

February 27, 2017

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Topsfield Planning Board Topsfield Town Hall 461 Boston Street, Unit E-6 Topsfield, MA 01983 Martha A. Morrison; Chairwoman RE: A&M Project #2165-01A Rolling Green Elderly Housing Development 470 Boston Street Topsfield, MA 01983 A&M Response Horsley Witten Review Letter

Dear Ms. Morrison:

On behalf of the applicant, Sarkis Development Company, Allen & Major Associates, Inc. (A&M), respectfully submits this peer review response letter regarding the comments generated by the Larson's (applicants neighbor) engineer Horsley Witten Group for the proposed Elderly Housing Development known as Rolling Green, located at 470 Boston Street, Topsfield, Massachusetts.

The following comments below were noted by Ms. Janet Carter Bernardo, P.E., of Horsley Witten Group on January 26, 2017, and pertain to general comments and comments related to the stormwater management; each comment is followed by A&M's response in **bold**.

COMMENTS:

1. The Planning Board and Conservation Commission should realize that as requested in HW's December 21, 2016 letter, the Applicant has revisited the area of woodlands under existing and proposed conditions. The October 2016 submission included a decrease of 0.17 acres of woods while the reissued submittal includes a decrease of 1.31 acres.

A&M Response: The error has been previously corrected and the post-development watershed plan and HydroCAD model have been updated accordingly.

2. As requested by Beals + Thomas and as listed in Volume 3, Chapter 1, page 28 of the Massachusetts Stormwater Handbook (MSH) a mounding analysis must be provided, Allen & Major Associates, Inc. (A&M) has stated that one will be provided. HW recommends that in the event that the mounding analysis requires a design change of one or more of the infiltration systems the hearings are not closed until the information has been provided and has been reviewed by Beals + Thomas.

Volume 3, Chapter 1, page 28:

Mounding analysis is required when the vertical separation from the bottom of an exfiltration system to seasonal high groundwater is less than four (4) feet and the recharge system is proposed to attenuate the peak discharge from a 10-year or higher 24-hour storm (e.g., 10-year,

civil & structural engineers

landscape

25-year, 50-year, or 100-year 24-hour storm). In such cases, the mounding analysis must demonstrate that the Required Recharge Volume (e.g., infiltration basin storage) is fully dewatered within 72 hours (so the next storm can be stored for exfiltration). The mounding analysis must also show that the groundwater mound that forms under the recharge system will not break out above the land or water surface of a wetland (e.g., it doesn't increase the water sheet elevation in a Bordering Vegetated Wetland, Salt Marsh, or Land Under Water within the 72-hour evaluation period).

A&M Response: A mounding analysis has been performed by New England Environmental (a division of SWCA, Incorporated). Please see attached information.

3. A&M have stated that the three study points will not result in a net increase in flow or volume during all analyzed storm events. However it appears that there will be an increase in flow during the 100-year storm event at Study Point (SP) 3 and at the down gradient property line. Furthermore as a simple analysis the HydroCAD model provided by the Applicant cumulatively adds the three study points together to evaluate the impacts at the down gradient property line. However to more accurately evaluate the flows at the property line, SP1 should be routed through the culvert to SP2 and then SP1 and SP2 should be routed through the onsite wetland system to SP3. In accordance with the MSH, Volume 1, Chapter 1, page 1, Standard 2: *Stormwater management systems shall be designed so that post-development peak discharge rates do not exceed pre-development peak discharge rates.* The concern is the potential flooding of the Larson property as well as the overtopping of North Street. As stated in HW's December 21, 2016 letter, the Larsons have witnessed the overtopping of North Street once in the past 20 years, indicating that the flooding of North Street does not happen often, however it can happen.

A&M Response: The HydroCAD model has been updated as suggested above. Refer to the drainage report revised through February 27, 2017, for the updated HydroCAD model as well as pre- and post-development peak flow data. In conclusion, the peak flows have been reduced in the post-development watershed, as compared to the pre-development watershed peak flows.

4. The Applicant has conducted additional soil testing within the footprint of UIS-1 in December 2016. TP#17 appears to be the controlling test hole with an Estimated Seasonal High Ground Water (ESHGW) at 56.5. In 1999 a number of test pits were conducted within the same area for a potential septic system and witnessed by the Town Engineer. It appears that the range of ESHGW within the same area in 1999 was between 62.3 and 62.5, considerably higher than the 2016 elevation. HW recommends that the Applicant explain the discrepancy in ESHGW.

A&M Response: The Soil Evaluations performed by Certified Soil Evaluators from Allen & Major Associates (A&M) in December of 2016, Eaglebrook Engineering & Survey, LLC in July 2016, New England Environmental (a division of SWCA, Incorporated), in December 2016, and portions of the Septic System Plan prepared by Hayes Engineering in 1999 were reviewed. It has been determined that the soils on-site are well drained, and water moves quickly and easily throughout the soil matrix. It was also determined that the soil testing near UIS-1 on the Drainage Plan yielded an estimated seasonal high groundwater table (ESHGW) at elevation 56.5, which corresponds to the adjacent wetland elevation of 57.0-58.0. Wetlands are often an indicator of the approximate ESHGW, as the wetlands are formed where the ESHGW breaks out from land. It should also be noted that three *independent* soil evaluators from Allen & Major Associates, Eaglebrook and New England Environmental, a division of SWCA, Incorporated, tested the soils on-site and all three attained

similar and consistent results. Based on currently accepted scientific standards including Title V Regulations, it is Allen & Major's professional opinion that the soil evaluations performed by Eaglebrook Engineering & Survey, LLC, New England Environmental (a division of SWCA), and A&M are correct in identifying the ESHGW elevation. See also attached response from Eaglebrook Engineering & Survey, LLC, which expounds further upon the test pit data.

5. A&M has stated that the use of Calcium Chloride will be reduced to only that which is necessary for public safety. The Larsons appreciate that concern for their drinking water supply and the wetlands at 470 Boston Street as well as 109 North Street have been considered. HW requests that a condition be included requiring the Applicant to minimize the amount of road salt utilized at 470 Boston Street to the maximum extent practicable.

A&M Response: The Applicant accepts the condition whereby calcium chloride will be used at the minimum amount practicable to maintain a safe environment at the proposed Elderly Housing Development.

6. The Applicant has located one of the proposed subsurface wastewater systems 10 feet from the Larson's property boundary. HW requests that the system be shifted further from the property boundary or relocated to the proposed reserve area. A 25 foot landscaped buffer zone is required per Section 3.16.C.1.q, and the subsurface system will limit the landscaping opportunities in this area.

A&M Response: See updated Sewage Disposal System plan set for septic redesign. Sheets SDS-1 and SDS 1.1 clearly show the redesigned septic system layout. In conclusion, the primary system has been moved further from the property line and the proposed reserve location allows for a landscaped buffer with proposed evergreen trees.

7. A&M has stated that the landscape screening materials are being reviewed. HW suggests that the proposed plantings along the western property boundary be replaced with conifer trees for a denser buffer in accordance with Section 3.16.E.2.

A&M Response: See updated landscape architecture plans by Brown + Sardina, particularly the Planting and Lighting Plan, Sheet L-1.

8. HW understands that the Applicant is seeking a waiver from the Conservation Commission to shift the final discharge point of FES 3. HW supports this relocation and appreciates the Applicant's cooperation in responding to the Larsons' concern.

A&M Response: The discharge location of FES-3 has been revised as shown on the design plans to a new location further away from the Larson property. The Applicant requests Conservation Commission approval for the updated location.

If you have any questions or comments, please do not hesitate to contact me at (781)-935-6889. We look forward to further discussing the project at the Topsfield Planning Board public hearing on March 7, 2017. Please provide A&M with the time and place of that public hearing.

A&M Project #2165-01A

Very truly yours,

ALLEN & MAJOR ASSOCIATES, INC.

Ryan Bianchetto, LEED AP Project Manager

Cc via email: Sarkis Development Company Eaglebrook Engineering & Survey, LLC Brown + Sardina New England Environmental (a division of SWCA, Incorporated) Beals + Thomas, Inc. Town of Topsfield Conservation Commission

Enclosures:

- 1) Revised Site Plans for Rolling Green Elderly Housing Development, prepared by Allen & Major Associates, revised through February 27, 2017
- Drainage Report, prepared by Allen & Major Associates and revised through February 27, 2017
- Additional Information prepared by New England Environmental, a division of SWCA, Incorporated
- 4) Soil Test Pit letter from Eaglebrook Engineering & Survey, LLC



Eaglebrook Engineering & Survey, LLC

Civil Engineers, Land Planners and Land Surveyors

February 27, 2017

Mr. John Sarkis Sarkis Development Company 2 Elm Square Andover, MA 01810

Re: 470 Boston Street Topsfield, MA

Dear Mr. Sarkis:

Eaglebrook Engineering & Survey has had the opportunity to review the request for clarification from Horsley Witten Group in their January 26, 2017 letter, comment number 4:

4. The Applicant has conducted soil testing within the footprint of UIS-1 in December 2016. TP#17 appears to be the controlling test hole with an Estimated Seasonal High Ground Water (ESHGW) at 56.5. In 1999 a number of test pits were conducted within the same area for a potential septic system and witnessed by the Town Engineer. It appears that the range of ESHGW within the same area in 1999 was between 62.3 and 62.5, considerably higher than the 2016 elevation. HW recommends that the Applicant explain the discrepancy in ESHGW.

We reviewed the Soil Evaluations performed by Eaglebrook Engineering & Survey in July 2016, the Soil Evaluations performed by Allen & Major Associates in December 2016, the Soil Evaluations performed by New England Environmental in December 2016, and excerpts of the Septic System Plan prepared by Hayes Engineering in 1999.

Eaglebrook Engineering & Survey conducted several deep hole and percolation tests throughout the site in July 2016 for the purposes of assessing the suitability of the on-site soils for subsurface sewage disposal systems associated with the proposed multi unit residential development. Our investigations determined the parent soils on-site at 470 Boston Street are coarse-to-fine, gravelly sandy ablation deposits. They are well drained with high permeability, demonstrated by the rapid percolation rates. The soil matrix is crudely stratified and uncompacted with a high percentage of open pores. Water moves quickly through the matrix. The rock aggregate contains a high percentage of both Iron and manganese and is highly weathered. As Certified Soil Evaluators, we are trained to



Eaglebrook Engineering & Survey, LLC

Civil Engineers, Land Planners and Land Surveyors

determine the estimated seasonal high groundwater based on areas of reddish to purple iron and manganese concentrations (oxidation) juxtaposed with areas of gray to green concentrations (reduction). It is crucial to see both the reduction and oxidation colors together.

In 1995 the State Environmental Code, Title V, was revised with major updates from the previous 1978 Code. One major revision was the methodology to determine the ESWGW. Prior to 1995, engineers and designers relied on observed groundwater elevations during anticipated seasonal high water periods of the year. Starting in 1995 the Massachusetts Department of Environmental Protection (MADEP) established the certified soil evaluator designation to train designers and engineers to evaluate soils based on redoximorphic features (oxidation and reduction concentrations). The initial years following the Code revision, Board of Health Agents and Soil Evaluators alike often estimated the ESHGW when any color was observed in the soil. The MADEP education program for Certified Soil Evaluators has progressed significantly in the past 15 years to better train soil evaluators to understand the geology of the site they're assessing, understand the physical as well as chemical processes associated with soil formation, and the relationship to seasonal high groundwater observations.

It is our interpretation that the colors observed in 1999 were a result of variegation and not the true seasonal high water table. Water that falls as precipitation moves quickly through the soil. The soil is wet to moist for a period of time and then returns to a lower moisture content as the supply of water is diminished. Due to the weathered nature of the rock content, it is highly susceptible to chemical erosion. As a result the iron and manganese is expressed as "rust" stains. Color variegations were observed in the soil evaluations performed by Eaglebrook in 2016, but are not an indication of Estimated Seasonal High Groundwater. These colors stood alone and were associated with individual rock fragments and were present at a depth just beneath the subsoil. Reduction colors were not associated with any of the oxidized colors until further depth in the test holes were reached indicating the elevation of ESHGW.

Additionally, in high permeability soils ESHGW generally normalizes to a consistent elevation since there are no restrictive soils to hold groundwater at a higher elevation. The soil testing in the vicinity of UIS-1 performed by Allen & Major Associates determined the ESWGW at elevation 56.5, which corresponds to the adjacent bordering vegetated wetland elevation of approximately 57.0-58.0.

Three individual soil evaluators conducted testing on the site in 2016 independently for different aspects of the site design and the data showed consistent results. It is our



Eaglebrook Engineering & Survey, LLC

Civil Engineers, Land Planners and Land Surveyors

professional opinion that the soil evaluations performed by Eaglebrook Engineering & Survey and Allen & Major Associates correctly determined the Estimated Seasonal High Groundwater elevation and the Estimated Seasonal High Groundwater elevation identified in the 1999 evaluation represents variegated colors of water moving down through the soil matrix as opposed to the phreatic water table rising to the surface.

If you have any questions please do not hesitate to contact me at any time. Thank you.

Sincerely, EAGLEBROOK ENGINEERING & SURVEY, LLC

Ch

Alexander Parker

Kenneth C. Knowles, P.E., CSE#1247 Principal Alexander F. Parker, CSE#1848



Hydrogeologic Investigation

Proposed Residential Development 470 Boston Street Topsfield, Massachusetts

Submitted: February 28, 2017

Prepared for:

Sarkis Development Company 2 Elm Square Andover, MA 01801

SWCA Project No. 039312.00



New England Environmental, Inc.

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February 28, 2017

John Sarkis Sarkis Development Company 2 Elm Square Andover, MA 01801

e-mail: <u>itsarkis@sarkisdevelopment.com</u>

RE: Hydrogeologic Investigation Proposed Residential Development 470 Boston Street Topsfield, Massachusetts

Dear Mr. Sarkis:

New England Environmental, a division of SWCA, Incorporated (NEE/SWCA), is pleased to provide you this report regarding a hydrogeologic evaluation of subsurface drainage conditions for the above-referenced site. The objective of the hydrogeologic investigation is to evaluate 1) subsurface soil and groundwater conditions, 2) the infiltration capacity of the soil, and 3) groundwater mounding relative to septic and stormwater infiltration systems proposed for the "Rolling Green" residential development. As part of the field investigation, three test pits were advanced to collect representative soil samples. Published information and on-site deep hole observations made by others were also reviewed to prepare a conceptual site model.

The Hantush equation was utilized to estimate groundwater mounding. The groundwater mounding calculations indicated that each of the three planned wastewater leaching facilities have minimal mounding with no adverse impact to the adjacent wetlands or to the planned infiltration pond. The stormwater infiltration basins also have little or no impact resulting from mounding. The groundwater model indicates that each of the basins completely drain the volume of a 25-year 24-hour storm event.

This report also addresses the following preliminary comments received by the Town of Topsfield:

Horsley Witten (HW) comment:

As requested by Beals + Thomas and as listed in Volume 3, Chapter 1, page 28 of the Massachusetts Stormwater Handbook (MSH) a mounding analysis must be provided, Allen & Major Associates, Inc. (A&M) has stated that one will be provided. HW recommends that in the event that the mounding analysis requires a design change of one or more of the infiltration systems the hearings are not closed until the information has been provided and has been reviewed by Beals + Thomas.

MSH Volume 3, Chapter 1, page 28:

Mounding analysis is required when the vertical separation from the bottom of an exfiltration system to seasonal high groundwater is less than four (4) feet and the recharge system is proposed to attenuate the peak discharge from a 10-year or higher 24-hour storm (e.g., 10-year, 25-year, 50-year, or 100-year 24-hour storm). In such cases, the

mounding analysis must demonstrate that the Required Recharge Volume (e.g., infiltration basin storage) is fully dewatered within 72 hours (so the next storm can be stored for exfiltration). The mounding analysis must also show that the groundwater mound that forms under the recharge system will not break out above the land or water surface of a wetland (e.g., it doesn't increase the water sheet elevation in a Bordering Vegetated Wetland, Salt Marsh, or Land Under Water within the 72-hour evaluation period).

Beals & Thomas (B+T) Comment:

Standard 3 of the Handbook requires a mounding analysis for infiltrative best management practices (BMPs) when separation to groundwater is less than four (4) vertical feet. The Applicant acknowledges only a two (2) foot vertical separation exists, but states that these BMPs are not attenuated systems and the mounding analysis is not required. However, for both systems, the inflow rate is greater than the outflow rate, indicating that some level of attenuation is occurring within each system. We request that the Applicant clarify the design intent of these BMPs relative to the required mounding analysis and provide the noted calculations as applicable.

Applicant's Response: The Applicant has contracted with New England Environmental to conduct such mounding analyses for the applicable systems and will provide such report upon completion.

Current B+T Response: We acknowledge the response provided by the Applicant and reiterate the intent of our original comment pending the submission of the outstanding mounding analysis. We recommend that full compliance with Handbook and Standard 3 be made a condition of the decision if the Project is approved prior to the submission of the required analysis.

NEE response to above HW and B+T Comments: The stormwater infiltration designs meet the above MSH criteria for a 25-year 24-hour rainfall event.

If you have any questions or comments, please contact us at 413-256-0202.

Sincerely, NEW ENGLAND ENVIRONMENTAL a division of SWCA, Incorporated

John P. Jemsek, Ph.D., LSP Remediation Team Lead/Sr. Hydrogeologist

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ATTACHMENTS

Attachment A – Test Pit Logs Attachment B – Grain Size Analyses Attachment C – Groundwater Mounding Worksheets

1.0 INTRODUCTION

New England Environmental, a division of SWCA, Incorporated (NEE/SWCA) has conducted a hydrogeologic evaluation of the proposed residential development at 470 Boston Street, Topsfield, Massachusetts. The study was conducted on behalf of Sarkis Development Company (Sarkis). The objective of the hydrogeologic investigation is to evaluate the following:

- 1) Subsurface soil and groundwater conditions,
- 2) Infiltration capacity of the soil, and
- 3) Groundwater mounding relative to proposed subsurface wastewater and stormwater infiltration systems.

NEE advanced three test pits to collect soil, evaluate soil hydraulic properties, and confirm overburden conditions. Deep hole observations and percolation tests logs prepared by others were also reviewed, as well as published information. Utilizing the Frimpter (1981) method, seasonal high water table estimates were compared to regional groundwater conditions to assess the hydrogeologic setting. The groundwater information was utilized to develop a conceptual site model, and estimate aquifer properties applicable to the site. Potential groundwater mounding for the three subsurface wastewater disposal systems and various stormwater infiltration systems proposed was then estimated utilizing the Hantush (1967) equation. Results were compared to applicable local and state regulatory guidelines and codes. The work was completed in general accordance with our proposal dated November 30, 2016 and subsequent authorization on December 1, 2016.

Three attachments have been provided within this document: one concerning the available test pit logs (**Attachment A**), one regarding grain size analyses conducted on representative soil samples (**Attachment B**), and one providing sample worksheets concerning the groundwater mounding (**Attachment C**). Also provided with this report are figures and summary tables.

2.0 SITE AND PROJECT DESCRIPTION

The project site is located off Boston Street (Route 1) in Topsfield, Massachusetts (see **Figure 1 Site Locus** and **Figure 2 Aerial Photo**). The site is about 16.3 acres and is currently developed with a single-family home and out-buildings. Boston Street borders the site to the east, wetlands and a church to the north, wetlands and the Ipswich town line to the west, and a woodlands and an apartment complex to the south. An open field exists in the central and western portion of the site. The surface topography at the site is undulating, ranging from elevation (EI) 83 feet in the southern portion of the Site to about EI 57 feet in the wetlands.

Proposed development of the site includes construction of a 15 duplexes (30 units total) for elderly housing, along with pavement and stormwater appurtenances. Three community subsurface sewage disposal systems (SDS) totaling 4,500 gallons per day (GPD) are planned for the development. The individual systems will include a 1,200 GPD system (western, SDS #1), 1,800 GPD system (central, SDS #3) and a 1,500 GPD system (eastern, SDS #2) system. Additionally, stormwater runoff will be collected and directed to a series of nine (9) underground infiltration systems (UIS-1 through UIS-9) and an infiltration pond in the center of the site (D-1).

Eaglebrook Engineering & Survey, LLC (Eaglebrook) of Danvers, Massachusetts advanced 12 deep hole observations (test pits) at the site in July 2016 to estimate seasonal high water table (SHWT) elevations in the vicinity of the proposed SDS units. The SHWT was determined to be

7.5 to 10 feet below grade in the vicinity of the proposed SDS units, based on soil mottling evidence. The SHWT depths generally increased from east to west on the upland flat where the development is planned. Eaglebrook also conducted two (2) percolation tests in each of the three system areas, and obtained rates ranging from 1.0 minutes per inch (MPI) for the western system to 7.33 MPI for the eastern system. Eaglebrook's Soil Suitability Assessment Report utilizing DEP Form 11 is provided within **Attachment A**, and includes the logs for TP16-1 through TP16-4 (SDS#1), TP16-5 through TP16-8 (SDS#3), and TP16-9 through TP16-12 (SDS#2), along with the summary of percolation tests 1 through 6.

Allen & Major Associates, Inc. (Allen & Major) of Woburn, Massachusetts also advanced test pits to support a general engineering evaluation, including the design of the planned stormwater infiltration units and pond. Test pits TP-1 through TP-9 were advanced in July 2016 and TP-10 through TP-17 were advanced in December 2016. The test pit logs are summarized on DEP Form 11s (see **Attachment A**).

The location of the test pits and infiltration features are shown on **Figure 3 Subsurface Exploration Location Plan**, and are based on plans prepared by Allen & Major as well as Eaglebrook.

3.0 FIELD AND LAB PROGRAM

3.1 Test Pit Observations

NEE monitored a subsurface exploration program which included the advancement of three test pits, one each in the vicinity of SDS #1 through SDS #3. Test pits SWTP-1 through SWTP-3 were coordinated with Allen & Major on December 9, 2016, utilizing an excavator operated by T.W. Excavating. Five (5) representative soil samples were collected and submitted for grain size analyses at the UMass Extension Soil and Nutrient Testing Laboratory utilizing sieve and hydrometer methodology. The test pit logs are provided in **Attachment A**, and the grain size analyses in **Attachment B**. The locations were estimated from existing site features and provided on **Figure 3**.

Soil at the site consists of a veneer of outwash deposit soils (Sand) over a crudely stratified ablation till (Sand and Gravel). Refusal was met at 14.5 feet, 12 feet and 6 feet at SWTP-1 through SWTP-3, respectively. The shallow refusal at 6 feet below grade at SWTP-3 was assumed to be a result of angular boulders, as adjacent test pits by Eaglebrook did not meet refusal at depths greater than 12 feet below grade. Groundwater was observed to be at or below 12 feet below grade on the day of the observations.

3.2 Hydraulic Conductivity Estimates

3.2.1 Grain-size Analysis

The representative soil samples from several soil strata encountered at the site were analyzed for grain-size distribution using sieve and/or hydrometer methods. The grain-size distribution curves have been included in **Attachment B**. An empirical relationship known as the Hazen Estimator allows the hydraulic conductivity of a granular soil to be estimated from the effective grain size, D_{10} , which is the effective grain size of the soil corresponding to 10 percent material finer by weight (see Lambe and Whitman, 1969). This relationship is considered reasonably

approximate for clean sands with D_{10} sizes between 0.1 and 3.0 millimeters. The hydraulic conductivity, k (centimeters/second [cm/sec]), of the material was calculated using the following equation:

$$k (cm/s) = D_{10}^2$$

where D_{10} is in millimeters. A summary of the hydraulic conductivity estimates are provided in **Table 1.** A mean hydraulic conductivity of 20 feet/day was obtained for the overburden soils. It should be noted that the range of hydraulic conductivity for soils is the greatest of any single soil parameter. Large deviations in hydraulic conductivity within a given soil deposit are common. Also, a number of uncontrollable factors influence the results of tests designed to measure hydraulic conductivity. Therefore, the results of these tests should be considered to be only an indicator of the soils hydraulic conductivity and not an absolute value. Additionally, actual hydraulic conductivity values are known to be 10 times or more in a horizontal direction versus the vertical direction, especially in the presence of stratification.

3.2.2 Percolation Tests

The percolation tests conducted by Eaglebrook were utilized as a second line of evidence for estimation of hydraulic conductivity of the overburden soils at the site. The empirical equation of Fritton et al. (1986) was utilized for this purpose. As shown in **Table 2**, the estimated hydraulic conductivity for the three SDS areas range from about 7 to 13 feet/day, which is consistent with the Hazen estimate provided in **Table 1**.

3.3 Seasonal High Groundwater Table Estimate with Frimpter Method

An estimate of the SHWT has been provided on the Eaglebrook and Allen & Major test pit logs utilizing the soil redoximorphic or mottling observations. For the Allen & Major test pits, the elevation for the SHWT is also shown as SHGWE on **Figure 3**. The edge of the wetland boundary also represents an estimate of the SHWT.

An independent estimate of the SHWT was made by employing methods developed by the US Geological Survey (USGS), specifically Frimpter (1981), where actual groundwater levels at a point in time are compared to USGS index wells. The USGS index wells have a long history of observations where seasonal trends are well-documented. The ratio of the potential water level rise at a site, to the potential water level rise at a USGS index well in a similar hydrogeologic setting, is found to be equal to ratio of the water level ranges at both locations.

NEE evaluated appropriate USGS index wells in the area and found the USGS Georgetown GCW 168 well to exhibit a similar range as the site. The Georgetown 168 well is located in an outwash deposit within a valley flat hydrogeologic setting. This setting is associated with relatively permeable soil with excellent drainage enhanced by local hydraulic relief provided by wetlands and/or surface water. As shown on **Table 3**, the Frimpter method suggests that the maximum range in water level at the site would be on the order of 4 feet, i.e., the SHWT would be about El 59 feet in the SDS #1 area, which is consistent with the SHGWE estimates in this area.

4.0 CONCEPTUAL SITE MODEL

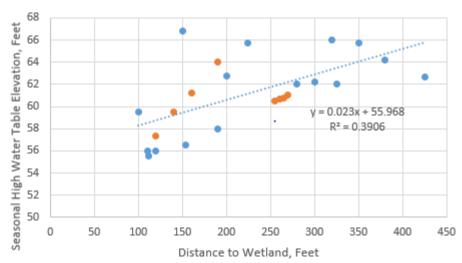
The surficial geology of the site has been mapped by USGS (Cuppels, 1969) and shows permeable outwash sand (Qgi) throughout the majority of the site. The outwash is projected to underlie wetlands to the north, and lie above a glacial till deposit that is exposed to the south of the site along the local topographic high (El 80 to 100 feet). The wetland to the north of the site has an intermittent drainage feature that discharges to Howlett Brook, located to the south of the Site. The presence of the wetlands to the north and Howlett Brook to the south provides hydraulic relief for groundwater flow at the site.



Insert 1 - Surficial Geology from USGS GQ-850 (Cuppels, 1969)

The outwash and glacial till has been shown to have high sand and gravel content and to be crudely stratified and having moderate permeability. Bedrock is not known to outcrop in the site area. USGS has mapped the bedrock formation to be the Topsfield granodiorite (Zen et al, 1983).

Groundwater at the site appears to flow to the north as higher groundwater elevations (and SHWT) trend to the south, and mirror higher ground surface elevations. Below, the SHWT elevation for the various test pits have been plotted versus distance to the wetland boundary. The overall trend suggests a 1 to 3 percent gradient exists. Variability properties within the outwash and till leads to scatter of the data. As expected, the groundwater data supports that the bordering wetland to the north serves as a constant head boundary, providing hydraulic relief for the site.



Groundwater Elevation Trend

Insert 2 - Allen & Major (blue) and Eaglebrook (orange) SHWT observations from test pitting

5.0 GROUNDWATER MOUNDING EVALUATION

5.1 Groundwater Model Selection and Calibration

Based on the local hydrogeologic observations, the saturated subsurface zone at the site is assumed to consist with an idealized one-layer water-table aquifer that is comprised of the outwash and stratified till underlying the site. Although the depth to bedrock is not known, upper bedrock in New England is typically fractured (horizontal sheeting and jointing) and also facilitates groundwater movement, and therefore contributes to some extent to the observed groundwater table surface. Based on the depth to groundwater observed in the test pits, the till unit is well-drained, as mapped by the Soil Conservation Services (see Allen & Major Drainage Report).

The drainage provided by Howlett Brook, its tributaries and the bordering wetlands act as a hydraulic relief for the site. A watershed divide is anticipated near the topographic divide located south of the site. The divide is located about 800 feet south of the wetland boundary located at and north of the site. These hydraulic features control the shape and slope of the unconfined groundwater table surface.

The Dupuit-Forcheimier (DF) equation for unconfined flow was utilized to estimate the effective transmissivity of the site. The DF equation that was utilized incorporates recharge. This creates a rise in the water table between two constant head boundaries of similar elevation. The rise creates a groundwater drainage divide or effective no-flow boundary between the two constant head boundaries. The available groundwater data at the site was incorporated as control points for the groundwater table surface, and the input to the DF equation was calibrated to match the control points. A recharge of 20 inches/year was assumed, and a constant head boundary of El 56 feet at the wetland boundary. The average water table heights were estimated to be about El 57.9, EI 60.7 and EI 63.7 at distances of 150, 300 and 450 south of the wetlands. The resultant transmissivity for the unconfined aquifer was estimated to be about 180 feet²/day, and this transmissivity was assumed within the subsequent mounding analyses (see Attachment C for DF model output). The average hydraulic conductivity assumed within the groundwater model is also consistent with the overburden estimates presented in Tables 1 and 2, and a nominal 20 The assumptions provide a "best-fit" estimate to the observed feet saturated thickness. groundwater data, with a root mean squared (RMS) effort of about 1.0 feet.

The Hantush (1967) equation for simulating groundwater mounding beneath an infiltration basin was utilized within the mounding estimates presented in the following sections. The spreadsheet provided by USGS SIR 2010-5102 (Carleton, 2010) was used for the calculations, and a sample spreadsheet output is provided within **Attachment C**. The Hantush model assumes a uniform application rate from the subject infiltration basin to the groundwater table. Horizontal groundwater flow within an unconfined aquifer, similar to the assumptions within the DF equation, is also assumed within the Hantush formula. A transmissivity of 180 feet²/day and a specific yield of 0.30 were assumed within each mounding calculation. The flow rates, duration and basin size was varied based on the design input provided by Eaglebrook for the SDS units and Allen & Major for the stormwater infiltration units.

5.2 Subsurface Sewage Disposal System Mounding Analysis

The input parameters and results for each SDS unit are provided in **Attachment C**. The mounding calculations indicate that a groundwater mound of less than 0.5 foot will result at each

of the SDS units. The mounding assumes a duration of 365 days, Title V flow rates for infiltration (maximum daily flow), and the SHWT as the ambient water table. Although the wetlands provide hydraulic relief to natural groundwater flow at the site, the mounding has no significant impact on the water table configuration adjacent to the wetlands, i.e., no breakout will occur adjacent to the wetlands.

The minimum bed bottoms elevations (bottom of sand) for each SDS, which is calculated as being 4 feet above the maximum mound height for each of the SSD systems, is summarized below:

Unit	Mounding Height (ft)	SHWT El (ft)	SHWT + Mounding El (ft)	Min Bed Bottom El (ft)
SDS#1 (west)	0.2	58.0	58.2	62.2
SDS#3 (central)	0.5	60.8	61.3	65.3
SDS#2 (east)	0.3	64.0	64.3	68.3

Pursuant to Topsfield's *R:I-2 Supplemental Regulations to 310 CMR 15.00 The State Environmental Code, Title 5, Section 11, Geohydrological Report on Clusters and Subdivisions,* the SDS systems are located in areas with adequate hydraulic capacity to accept the proposed design flows. Wastewater effluent plumes generated by the system are located at a sufficient upgradient distance to the wetlands, which are the nearest point of hydraulic relief, so that no impact to any environmental receptors is anticipated. The ground water mounding heights are predicted to be less than 0.5 feet, so that standard Title V design standards will be more than satisfactory for the proposed development.

5.3 Stormwater Basin Infiltration Capacity

Allen & Major has completed a design study for nine (9) subsurface stormwater infiltration beds (UIS-1 through UIS-9) and an infiltration pond (D-1) using run-off calculations for the Required Recharge Volume (RRV), 2-year, 10-year, 25-year and 100-year 24-hour rainfall events. The calculations were generated utilizing HydroCad. The Town of Topsfield requires that the proposed development must handle run-off from a 25-year 24-hour storm event. The basin designs for the proposed development are detailed within the Allen & Major's January 17, 2017 Drainage Report.

Application of the Hantush model to a stormwater infiltration problem required developing reasonable estimates for a uniform infiltration rate and duration for each storm. The *Massachusetts Stormwater Handbook Volume 3: Documenting Compliance with the Massachusetts Stormwater Management Standard* (MA Stormwater Handbook) refers to a 72-hour evaluation period for the RRV and 24-hour storms. Consequently, the infiltration rate was assumed to be equal to the total storage volume calculated by the HydroCad model, divided by the basin area, with the infiltration of the storage volume meted over the 24-hour storm period followed by a 72-hour evaluation period for a total 4 days. Therefore, each basin had a unique infiltration rate for the 25-year storm, with the infiltration rate ranging from 0.92 feet per day for UIS-1 (the largest UIS) to 0.25 feet per day for UIS-9 (the smallest UIS).

Using the Hantush model, maximum mounding heights were evaluated for the five loading scenarios at each UIS and the drainage pond. An aquifer with a conductivity of 8 ft/day, a thickness of 23 feet and a specific yield of 0.3 were assumed, consistent with the SDS mounding

evaluations. **Table 4** provides a summary of the calculated maximum mounding heights compared to vertical separation between the design bed bottom and the SHWT elevation.

As specified within the MA Stormwater Handbook, the 25-year storm event scenarios at each UIS met the dewatering criteria for the 72-hour observation period following the 24-hour rainfall event.

Mounding effects beyond the individual stormwater basins are minimal and are not sufficient to cause surface breakout. Hydraulic mounding related to the proposed infiltration pond D-1 also will not cause adverse impacts to the wetlands. Regarding the proximal relationship of the infiltration pond to septic system SDS#2, the pond infiltration will serve to temporarily increase the hydraulic gradient upgradient of SDS#2, eliminating any potential pathway between the SDS#2 effluent and the pond area. Assuming the pond bottom has no clogging, the water within the pond should dissipate within an additional day or two of the 72-hour evaluation period.

6.0 **FINDINGS**

The Hantush analytical model was utilized to calculate the potential effects of hydraulic mounding that would occur beneath proposed subsurface stormwater infiltration basins during various rainfall scenarios. Additionally, groundwater mounding beneath SDS units for three septic systems was evaluated.

Overall, the site has excellent hydraulic capacity to handle infiltration from engineered stormwater and wastewater disposal systems. This is based on the hydrogeologic setting featuring permeable sand and gravel, allowing well-drained upland conditions, with hydraulic relief facilitated by the adjacent wetland to the north of the site. Specifically, the results of the mounding analyses indicate the following:

- 1. Groundwater mounding beneath the proposed septic systems is minimal. Adding 0.5 foot of vertical separation in addition to the required 4 feet between the bed bottom and SHWT will achieve the performance standards expected under Title V.
- 2. For the stormwater infiltration systems, the 25-year 24-hour rainfall event is sufficiently handled in each of the nine (9) UIS;
- 3. No "breakout" of the effluent plume or stormwater discharge plume to the ground surface is predicted, either at the wetland boundary or any offsite location; and
- 4. No negative impact between SDS#3 and the D-1 pond is anticipated due to the hydraulic relationship of the features.

The one-dimensional unconfined groundwater flow models presented herein are a mathematical representation of the groundwater system and hydrogeologic setting. Due to the natural complexity of these entities, a model can only achieve a simplified representation of the future groundwater conditions, and therefore must be considered as a generalized screening tool for use in studying this Site. Furthermore, the calibration and validation of any model is limited by the availability and accuracy of field data and historical records of site activities.

7.0 **REFERENCES**

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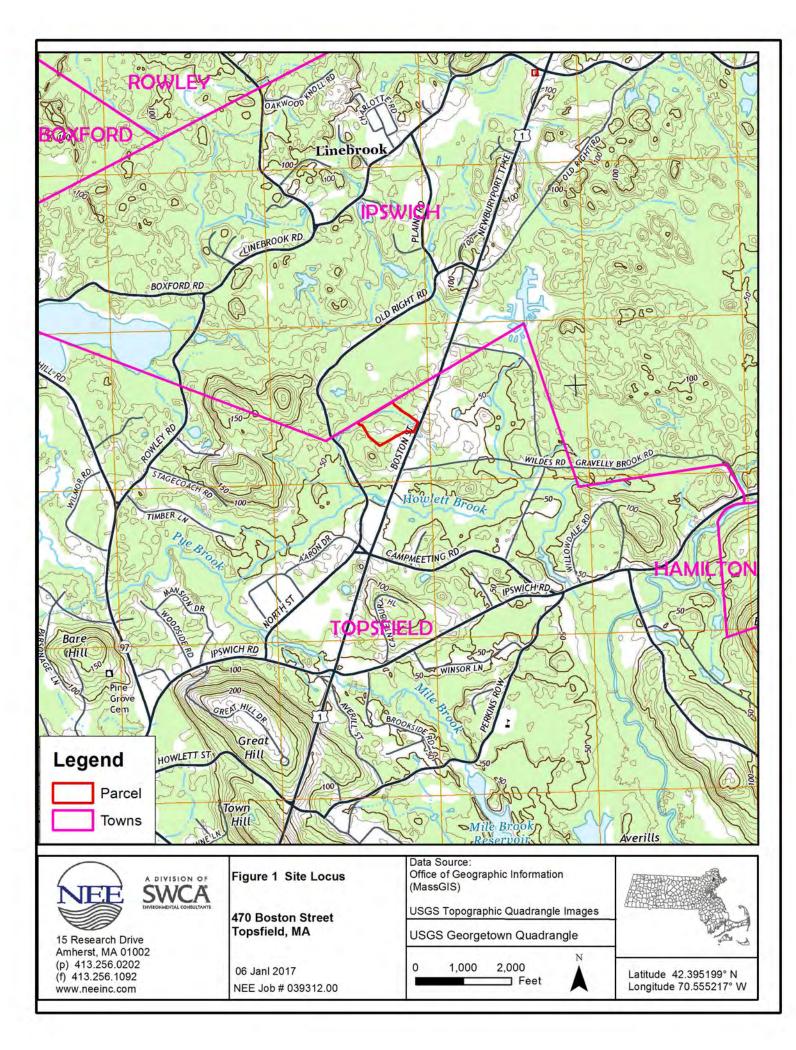


FIGURES

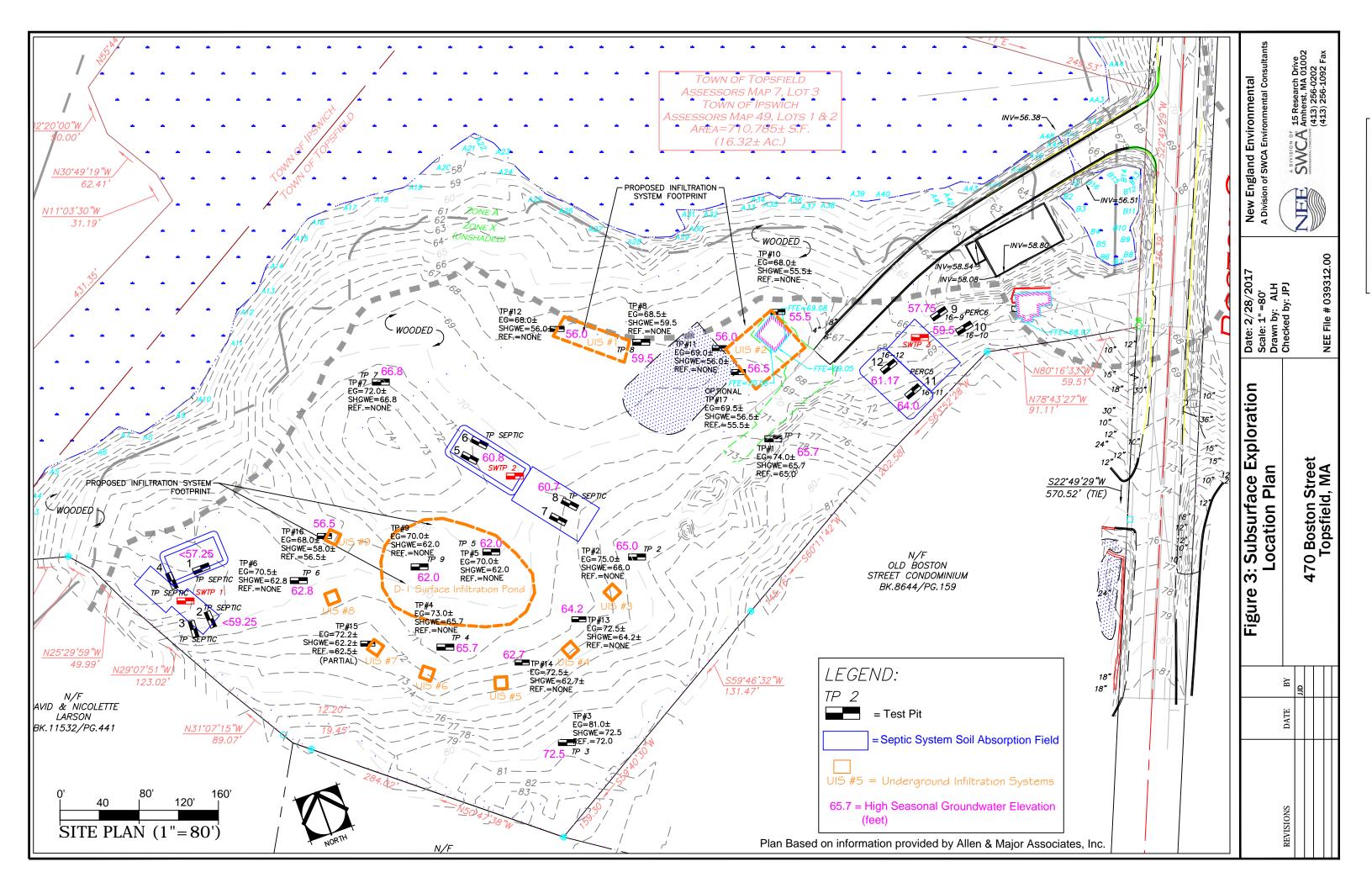
Figure 1	
Figure 2	
Figure 3	

	Site Locus
2	Aerial Photo
3	Subsurface Exploration Location Plan









TABLES

H

H

Table 1	Summary of Hazen Estimate for Hydraulic Conductivity
Table 2	Summary of Hydraulic Conductivity Estimation from Perc Tests
Table 3	Groundwater Level Estimation using the Frimpter Method
Table 4	Stormwater Mounding Calculations using Hantush Method



Table 1 - Summary of Hazen Estimate for Hydraulic Conductivity

470 Boston Street Topsfield, Massachusetts

	Depth		D10 Grain Size	Hydraulic Cor	nductivity Est.
Well ID	(feet)	Geologic Material	(mm)	(cm/s)	(ft/d)
Subsurface	e Disposal Sy	rstem #1			
SWTP-1	3-5	coarse SAND (outwash)	0.085	7.2E-03	20
SWTP-1	8-9	Gravelly loamy coarse SAND (ablation till)	0.065	4.2E-03	12
SWTP-1	11-12	Gravelly loamy coarse SAND (ablation till)	0.15	2.3E-02	64
Subsurface	e Disposal Sy	stem #3			
SWTP-2	10-12	Gravelly loamy coarse SAND (ablation till)	0.04	1.6E-03	4.6
Subsurface	e Disposal Sy	stem #2			
SWTP-3			0.015	2.2E-04	0.6
			arithmetic mean:	7.2E-03	20
			geometric mean:	3.0E-03	8

Notes:

Hazen Estimator = D10 (mm) ^2 (cm/s) used to estimate hydraulic conductivity from grain size results

Table 2 - Summary of Hydraulic Conductivity Estimation from Perc Tests

470 Boston Street Topsfield, Massachusetts

System #3 - PERC-1 at TP-2								
Percolation Rate	colation Rate mpi							
Hydraulic Conductivity	m/s	3.8E-05	cm/s	3.8E-03				
Hydraulic Conductivity	feet/day	10.8						
System #3 -PERC-4 at TP-5	5							
Percolation Rate	mpi	1		_				
Hydraulic Conductivity	m/s	4.4E-05	cm/s	4.4E-03				
Hydraulic Conductivity feet/day		12.7						
System #2 -PERC-6 at TP-1	LO							
Percolation Rate	mpi	7.33						
Hydraulic Conductivity	m/s	2.4E-05	cm/s	2.4E-03				
Hydraulic Conductivity feet/day		6.8						

Notes:

Emprical equation from D.D. Fritton, T.T. Ratvasky, and G.W. Petersen, "Determination of Saturated Hydraulic Conductivity from Soil Percolation Test Results", Soil Science of America Journal, Vol 50, No. 2, p 273-276, March 1986.

Table 3 - Groundwater Level Estimation using the Frimpter Method

C

470 Boston Street	SWCA Project Name
039312.00	SWCA Project Number
Topsfield, Massachusetts	Site Location

Frimpter Equation:

$$\frac{S-S_h}{OW-OW_{\max}} = \frac{S_r}{OW_r}$$

Where the following parameters are defined (water levels referenced to feet from ground surface):

S = Water level at site well Sh = Estimated depth to seasonal high water level at site well Sr = Estimated range of water level at site well (using topographic and lithologic setting) OW = Water level at USGS index well OWmax = Maximum water level at USGS index well OWr = Range of water level at USGS index well

Select USGS Well with similar topographic and lithologic setting as the site well:

	Georgetown Wel	ll GCW 168	Name of Selected USGS Index Well exhibiting similar				
Valley Flat T			Topographic Setting				
	Outwash		Lithologic Setting				
	2.25		OWmax (historic high water level)				
	4.40		OWr (LOW- OWmax or USGS-provided number)				
	5.22		OW				
	see below		Date of observation (OW)				
		Notes on OW:	5.34 on November 25, 2016 observed				
			4.86 on December 30, 2016 observed				
			estimate 5.22 for December 9, 2016				

Input data for site well:							
Test Pit SWTP-1	Site Well ID						
12/9/2016	Date of Observation						
68.25	Ground surface elevation						
56.25	Water level surface elevation						
12.00	S (calculated depth below ground surface)						
4.4	Sr						

Calculate estimated depth to seasonal high water level:

9.03	Sh
2.97	S-Sh
59.22	Estimated seasonal high water level elevation

Note: = input data = calculated

Notes:

See 1981 U.S. Geological Survey Water-Resource Investigation Report (WRI-80-1205) by Frimpter

Table 4 - Stormwater Mounding Calculations utilizing Hantush Method

470 Boston Street Topsfield, Massachusetts

							Hantush Mounding Calculation		
System I.D.	Basin L (Ft)	Basin W (Ft)	Finish Grade El (ft)	Bottom of Stone El (ft)	SHWT (ft)	Bottom El minus SHWT	Stormwater Volume (ft3) from HydroCad	3-Day Infiltration Rate (Ft/d)	Maximum Mounding Height (Ft)
Required R	echarge Vo	lume							
UIS-1	69	50	68	60	56.5	3.5	965	0.07	0.3
UIS-2	73.5	16	68.2	61.5	59.5	2	837	0.18	0.3
UIS-3	10	10	76.4	71.7	63.8	7.9	195	0.49	0.1
UIS-4	10	10	77.2	72.5	67.9	4.6	195	0.49	0.1
UIS-5	10	10	77.8	73.1	69	4.1	195	0.49	0.1
UIS-6	10	10	77	72.3	65.7	6.6	195	0.49	0.1
UIS-7	10	10	76.5	71.8	64.5	7.3	195	0.49	0.1
UIS-8	10	10	75.8	71.1	62.8	8.3	195	0.49	0.1
UIS-9	10	10	74.9	71.3	64	7.3	195	0.49	0.1
D-1	69	69	66	66	62	4	2,959	0.16	0.8
2-Year 24-h	nour Rainfal	I							
UIS-1	69	50	68	60	56.5	3.5	4,105	0.30	1.2
UIS-2	73.5	16	68.2	61.5	59.5	2	1,100	0.23	0.4
UIS-3	10	10	76.4	71.7	63.8	7.9	130	0.33	0.1
UIS-4	10	10	77.2	72.5	67.9	4.6	134	0.34	0.1
UIS-5	10	10	77.8	73.1	69	4.1	136	0.34	0.1
UIS-6	10	10	77	72.3	65.7	6.6	137	0.34	0.1
UIS-7	10	10	76.5	71.8	64.5	7.3	136	0.34	0.1
UIS-8	10	10	75.8	71.1	62.8	8.3	136	0.34	0.1
UIS-9	10	10	74.9	71.3	64	7.3	81	0.20	0.1
D-1	69	69	66	66	62	4	7,349	0.39	1.8
10-Year 24	-hour Rainfa	all							
UIS-1	69	50	68	60	56.5	3.5	8,986	0.65	2.5
UIS-2	73.5	16	68.2	61.5	59.5	2	1,956	0.42	0.7
UIS-3	10	10	76.4	71.7	63.8	7.9	135	0.34	0.1
UIS-4	10	10	77.2	72.5	67.9	4.6	141	0.35	0.1
UIS-5	10	10	77.8	73.1	69	4.1	144	0.36	0.1
UIS-6	10	10	77	72.3	65.7	6.6	146	0.37	0.1
UIS-7	10	10	76.5	71.8	64.5	7.3	144	0.36	0.1
UIS-8	10	10	75.8	71.1	62.8	8.3	144	0.36	0.1
UIS-9	10	10	74.9	71.3	64	7.3	91	0.23	0.1
D-1	69	69	66	66	62	4	13,275	0.70	3.2

Table 4 - Stormwater Mounding Calculations utilizing Hantush Method (cont.)

470 Boston Street Topsfield, Massachusetts

							Hantush N	Mounding Calc	ulation
System I.D.	Basin L (Ft)	Basin W (Ft)	Finish Grade El (ft)	Bottom of Stone El (ft)	SHWT (ft)	Bottom El minus SHWT	Stormwater Volume (ft3) from HydroCad	4-Day Infiltration Rate (Ft/d)	Maximum Mounding Height (Ft)
25-Year 24	-hour Rainfa	all					-		
UIS-1	69	50	68	60	56.5	3.5	12,660	0.92	3.4
UIS-2	73.5	16	68.2	61.5	59.5	2	2,548	0.54	0.8
UIS-3	10	10	76.4	71.7	63.8	7.9	137	0.34	0.1
UIS-4	10	10	77.2	72.5	67.9	4.6	146	0.37	0.1
UIS-5	10	10	77.8	73.1	69	4.1	151	0.38	0.1
UIS-6	10	10	77	72.3	65.7	6.6	154	0.39	0.1
UIS-7	10	10	76.5	71.8	64.5	7.3	151	0.38	0.1
UIS-8	10	10	75.8	71.1	62.8	8.3	151	0.38	0.1
UIS-9	10	10	74.9	71.3	64	7.3	99	0.25	0.1
D-1	70	70	66	66	62	4	14,288	0.73	3.4
100-Year 2	4-hour Rain	fall							
UIS-1	69	50	68	60	56.5	3.5	13,284	0.96	3.6
UIS-2	73.5	16	68.2	61.5	59.5	2	2,860	0.61	1
UIS-3	10	10	76.4	71.7	63.8	7.9	141	0.35	0.1
UIS-4	10	10	77.2	72.5	67.9	4.6	153	0.38	0.1
UIS-5	10	10	77.8	73.1	69	4.1	159	0.40	0.1
UIS-6	10	10	77	72.3	65.7	6.6	163	0.41	0.1
UIS-7	10	10	76.5	71.8	64.5	7.3	159	0.40	0.1
UIS-8	10	10	75.8	71.1	62.8	8.3	159	0.40	0.1
UIS-9	10	10	74.9	71.3	64	7.3	111	0.28	0.1
D-1	72	72	66	66	62	4	16,629	0.80	3.7

Notes:

L = Length, W= Width, El = Elevation, SHWT = Seasonal High Water Table estimate

Hantush (1967) equation for groundwater mounding beneath an infiltration basin utilized

Basin dimension, elevations and HydroCad stormwater flow volumes taken from January 17, 2017 Drainage Report prepared by Allen & Major Associates, Inc.

ATTACHMENT A

Test Pit Logs

H

H



SOIL SUITABILITY ASSESSMENT REPORT COMMONWEALTH OF MASSACHUSETTS TOPSFIELD, MASSACHUSETTS

SOIL EVALUATION FOR NEW CONSTRUCTION OF ON-SITE SUBSURFACE DISPOSAL SYSTEMS

SITE INFORMATION

Topsfield Assessor's Parcel ID: 7-3

Street Address: 470 Boston StreetTown: TopsfieldState: MassachusettsZip Code: 01983County: EssexLand Use: Undeveloped; open meadowLatitude: $\sim 42^{\circ} 39^{\circ} 50.1^{"}N$ Longitude: $\sim 70^{\circ} 55^{\circ} 51.1^{"}W$

PUBLISHED SOIL DATA AND MAP UNIT DESCRIPTION

Physiographic Division: <u>Appalachian Highlands</u> Physio. Province: <u>New England</u> Physio. Section: <u>Seaboard lowland section</u>
NRCS/USDA web soil survey: <u>Essex County, Massachusetts, Northern part</u> Map Scale: <u>1:300</u>'
Soil map unit: <u>420B - Canton fine sandy loam (coarse loamy over sandy, mixed, mesic, Typic Dystrochrepts), 03-08% slopes</u>
Soil temperature regime: <u>Mesic</u> Soil moisture regime: <u>Udic</u> Drainage Class: <u>Well drained</u> Hydrologic Soil Group: <u>A</u>
Ksat: <u>High (2.00 - 6.00 in/hr)</u> Available water capacity: <u>Low (~4.4")</u> Soil hydric/ upland: <u>Upland</u>
Depth to restrictive feature: <u>Variable depths to bedrock</u> Frequency of flooding: <u>None</u> Frequency of ponding: <u>None</u>
Soil limitations: <u>Moderate permeability, gravelly substratum, variable seasonal groundwater table, shallow to bedrock in areas.
</u>

CLOSEST USGS WELL MEASUREMENTS and WETLAND AREA

Current Water Resource Condition (USGS): <u>Well Site # 423115071032001- MA-WAW 38 Wakefield, MA</u> Middlesex County, Massachusetts, Hydrologic Unit 01090001 Latitude: <u>~42° 31 '00.2" N</u> Longitude: <u>~71° 02' 54.4" W</u> Well depth: <u>25.5 feet</u> Borehole depth: <u>28.2 feet</u> Land surface altitude: <u>80.00 feet above NGVD29</u> Most recent data value: <u>7.94' on 7/04/16 (depth to water level in feet below land surface)</u>. Range: <u>Much below normal</u> National Wetland Inventory Map: <u>NA</u> Wetlands Conservancy Program: <u>NA</u> Bordering vegetative wetland: <u>>100' feet</u>

SURFICIAL & BEDROCK GEOLOGY:

Surficial geology: <u>Geological Quadrangle Map</u>, <u>Surficial Geology – Georgetown Quadrangle</u>, <u>Mass</u>, <u>1958 – 1959</u> <u>Ogi: Deposits in the Ipswich River area</u>; Light-brown to light-gray, medium, well sorted sand in the valleys of the Ipswich River and its tributaries in the southern part of the quadrangle. Sandy deposits underlain by lodgment till. Geomorphic landform: <u>Kame plain</u> Landform position (2D): <u>Foot slope</u> Landform position (3D): <u>Baseslope</u> Slope aspect: <u>Easterly</u> Slope gradient: <u>~00-03%</u> Down slope shape: <u>Concave</u> Across slope shape: <u>Concave</u> Slope complexity: <u>Simple</u> Bedrock outcropping in vicinity: <u>Not observed</u> Glacial erratics in vicinity: <u>None observed</u> Bedrock Type: <u>Topsfield granodiorite – Gray to gray-green</u>, porphyritic granodiorite containing blue quartz; cataclastically foliated.

TP16-1 DEEP OBSERVATION HOLE

470 Boston Street, Topsfield, Massachusetts

Date:Wednesday, July 06, 2016Time: 08:15Weather:Sunny, dry, ~85°F, Northeast breezePosition on landscape:BackslopeSlope aspect:NorthwesterlyLand Cover:Meadow grassProperty line:10⁺ feetDrainage way:50⁺ feetDrinking water well:100⁺ feetWetlands:100⁺ feetIpswich River:400⁺ feetOpen water body:400⁺ feetAbutting septic system:NA

SOIL PROFILE ► TP16-1

Depth below land surface (inches)	Soil Horizon/ Layer	Soil Texture (USDA/ NRCS)	Soil Color (Munsell)	Redoxomorphic Features/ ESHGWT	Consistence, grade, size, structure, grain size, soil moisture state, roots, horizon boundary, clasts, stratification, artifacts, restrictive features, etc.
00 → 14"	A _P	Sandy Loam	10YR 3/2 very dark grayish brown	none observed	Very friable; moderate-grade fine to medium subangular granular structure; weak cohesive matrix; fine grained mineral content; slightly damp; common fine to medium roots; free of clasts; clear wavy boundary.
14 → 34"	B_{W}	Sandy Loam	2.5Y 6/6 olive yellow	none observed	Very friable; weak-grade fine to medium angular blocky structure; weak cohesive matrix; gritty; mixed medium to mostly fine grained mineral content; damp; ~05% subrounded gravel content; diffuse wavy boundary.
34 → 88"	C ₁	Sand gravelly	10YR 4/4 dark yellowish brown		Loose; structurless; unstable; mixed fine to coarse grained sand; crudely stratified and well graded; ~10 -15% rounded to subrounded gravel content of mixed lithology; few strong red variegated iron stains on clasts and along bedding planes; stratified beds slightly dipping to the East; no refusal at test hole depth.
88 → 119"	C ₂	Sandy Loam gravelly	7.5YR 5/3 brown	none observed	Friable; moderate-grade medium-to-coarse subangular platy structure; mixed fine to coarse grained mineral content; well- graded; moderately compact matrix; crudely stratified with minor imbrication of clasts; somewhat silty; damp; ~15-20% scattered subangular-to-subrounded gravel and ~10% scattered subangular- cobble content of mixed lithology; clasts somewhat tightly nested in matrix; compactness slightly increases with depth; no bedrock contact at test hole depth and no apparent water observed.

Depth to bedrock: <a>119"Seasonal High Groundwater Table: <a>119"Apparent water table: <a>119"420B - Canton fine sandy loam (coarse loamy over sandy, mixed, mesic, Typic Dystrochrepts), 03-08% slopes

TP16-1 DEEP OBSERVATION HOLE

470 Boston Street, Topsfield, Massachusetts

DEPTH TO APPARENT/ PHREATIC GROUNDWATER TABLE: None Observe	d
Apparent water seeping from pit face: (below land surface) Depth to stabilized apparent water: (be	_
Soil moisture state: Damp	
ESTIMATED SEASONAL HIGH GROUNDWATER TABLE: None Observed	
Depth of Estimated Seasonal High Groundwater Table: (below land surface)	
Type: Abundance: Size: Contrast:	
Shape: Moisture state: Location:	
Hardness: Boundary: Concentration color: Reduction color:	olor:
DETERMINATION OF HIGH GROUNDWATER ELEVATION	
Observed depth to stabilized phreatic water: inches below grade	
Observed water weeping from side of deep hole: inches below grade	
Observed depth to redoximorphic features: inches below grade	
<u>DEPTH OF NATURALLY OCCURRING PERVIOUS MATERIAL</u> : ► <u>8.75 feet</u>	
Depth of naturally occurring pervious material in TP16-1 Upper boundary: <u>14</u> "	
Lower boundary: <u>119"</u>	
<u>Certification</u>	
I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to	
evaluations and that the above analysis has been performed by me consistent with the required training, expertise and e 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Fo	
with 310 CMR 15.017.	
Alexander F. Parker Certified Soil Evaluator #1848	une 1998
	Date of Soil Evaluator Certification
	07/06/16
Town of Topsfield witness	Date of soil testing

TP16-2 DEEP OBSERVATION HOLE

470 Boston Street, Topsfield, Massachusetts

Date: Wednesday, July	<u>06, 2016</u>	Time: <u>08:30</u>	Weather: Su	<u>inny, dry, ~85°F, N</u>	Jortheast breeze
Position on landscape: <u>E</u>	<u> Backslope</u>	Slope aspect: <u>N</u>	orthwesterly	Land Cover: M	eadow grass
Property line: <u>10⁺ feet</u>	Drainage	way: <u>50⁺ feet</u>	Drinking wate	er well: 100^+ feet	
Wetlands: <u>100⁺ feet</u>	Ipswich Ri	iver: <u>400⁺ feet</u>	Open water b	ody: <u>400⁺ feet</u>	Abutting septic system: <u>NA</u>

SOIL PROFILE ► TP16-2

Depth below land surface (inches)	Soil Horizon/ Layer	Soil Texture (USDA/ NRCS)	Soil Color (Munsell)	Redoxomorphic Features/ ESHGWT	Consistence, grade, size, structure, grain size, soil moisture state, roots, horizon boundary, clasts, stratification, artifacts, restrictive features, etc.
00 → 08"	A _P	Sandy Loam	10YR 3/2 very dark grayish brown	none observed	Very friable; moderate-grade fine to medium subangular granular structure; weak cohesive matrix; fine grained mineral content; slightly damp; common fine to medium roots; free of clasts; clear wavy boundary.
08 → 26"	Bw	Sandy Loam	2.5Y 6/6 olive yellow	none observed	Very friable; weak-grade fine to medium angular blocky structure; weak cohesive matrix; gritty; mixed medium to mostly fine grained mineral content; damp; ~05% subrounded gravel content; diffuse wavy boundary.
26 → 117"	С	Sandy Loam gravelly	7.5YR 5/3 brown	none observed	Friable; moderate-grade medium-to-coarse subangular platy structure; mixed fine to coarse grained mineral content; well- graded; moderately compact matrix; crudely stratified with minor imbrication of clasts; somewhat silty; damp; ~15-20% scattered subangular-to-subrounded gravel and ~10% scattered subangular- cobble content of mixed lithology; clasts somewhat tightly nested in matrix; compactness slightly increases with depth; no bedrock contact at test hole depth and no apparent water observed.

Depth to bedrock: $\geq 117^{"}$ Seasonal High Groundwater Table: $\geq 117^{"}$ Apparent water table: $\geq 117^{"}$ 420B - Canton fine sandy loam (coarse loamy over sandy, mixed, mesic, Typic Dystrochrepts), 03-08% slopes

TP16-2 DEEP OBSERVATION HOLE

470 Boston Street, Topsfield, Massachusetts

DEPTH TO APPARENT/ PHREATIC GROUNDWATER TABLE: None Observed						
Apparent water seeping from pit face: (below land surface) Depth to stabilized apparent water: (below land surface)						
Soil moisture state: Damp						
Son moisture state. <u>Dump</u>						
ESTIMATED SEASONAL HIGH GROUNDWATER TABLE: None Observed						
Depth of Estimated Seasonal High Groundwater Table:						
Type: Abundance: Size: Contrast:						
Shape: Moisture state: Location:						
Hardness: Boundary: Concentration color: Reduction c	polor.					
Concentration color Reduction e						
DETERMINATION OF HIGH GROUNDWATER ELEVATION						
Observed depth to stabilized phreatic water: inches below grade						
Observed water weeping from side of deep hole: inches below grade						
Observed depth to redoximorphic features: inches below grade						
<u>DEPTH OF NATURALLY OCCURRING PERVIOUS MATERIAL:</u> \blacktriangleright 8.25 feet						
Depth of naturally occurring pervious material in TP16-2 Upper boundary: <u>08</u> "						
Lower boundary: <u>117</u> "						
Certification						
	1 /					
I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in						
310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.017.						
Alexander F. Parker Certified Soil Evaluator #1848	June 1998					
Printed name of evaluator & certification number	Date of Soil Evaluator Certification					
Mr. John Coulon, Topsfield Director of Public Health	07/06/16					
Town of Topsfield witness	Date of soil testing					

TP16-3 DEEP OBSERVATION HOLE

470 Boston Street, Topsfield, Massachusetts

Date: Wednesday, July 06, 2	2016 Time: <u>08:46</u>	Weather: <u>Sunny, dry, ~85°F, N</u>	lortheast breeze
Position on landscape: Backs	slope Slope aspect: <u>No</u>	rthwesterly Land Cover: M	eadow grass
Property line: 10^+ feet D	Drainage way: <u>50⁺ feet</u>	Drinking water well: 100^+ feet	
Wetlands: <u>100⁺ feet</u> Ips	swich River: <u>400⁺ feet</u>	Open water body: 400^+ feet	Abutting septic system: <u>NA</u>

SOIL PROFILE ► TP16-3

Depth below land surface (inches)	Soil Horizon/ Layer	Soil Texture (USDA/ NRCS)	Soil Color (Munsell)	Redoxomorphic Features/ ESHGWT	Consistence, grade, size, structure, grain size, soil moisture state, roots, horizon boundary, clasts, stratification, artifacts, restrictive features, etc.
00 → 10"	A _P	Sandy Loam	10YR 3/2 very dark grayish brown	none observed	Very friable; moderate-grade fine to medium subangular granular structure; weak cohesive matrix; fine grained mineral content; slightly damp; common fine to medium roots; free of clasts; clear wavy boundary.
10 → 24"	B_{W}	Sandy Loam	2.5Y 6/6 olive yellow	none observed	Very friable; weak-grade fine to medium angular blocky structure; weak cohesive matrix; gritty; mixed medium to mostly fine grained mineral content; damp; ~05% subrounded gravel content; diffuse wavy boundary.
24 → 102"	С	Sandy Loam gravelly	7.5YR 5/3 brown	none observed	Friable; moderate-grade medium-to-coarse subangular platy structure; mixed fine to coarse grained mineral content; well- graded; moderately compact matrix; crudely stratified with minor imbrication of clasts; somewhat silty; damp; ~15-20% scattered subangular-to-subrounded gravel and ~10% scattered subangular- cobble content of mixed lithology; clasts somewhat tightly nested in matrix; compactness slightly increases with depth; no bedrock contact at test hole depth and no apparent water observed.
102"	R	Hard Bedrock			Continuous and coherent contact with granodioritic rock 300 Series excavator unable to proceed deeper

 Depth to bedrock: <u>102</u>"
 Seasonal High Groundwater Table: <u>>102</u>"
 Apparent water table: <u>>102</u>"

 <u>420B - Canton fine sandy loam (coarse loamy over sandy, mixed, mesic, Typic Dystrochrepts), 03-08% slopes</u>

TP16-3 DEEP OBSERVATION HOLE

470 Boston Street, Topsfield, Massachusetts

DEPTH TO APPARENT/ PHREATIC GROUNDWATER TABLE: None Observed	<u>d</u>
Apparent water seeping from pit face:	low land surface)
Soil moisture state: Damp	
ESTIMATED SEASONAL HIGH GROUNDWATER TABLE: None Observed	
Depth of Estimated Seasonal High Groundwater Table: (below land surface)	
Type: Size: Contrast:	
Shape: Moisture state: Location:	
Hardness: Boundary: Concentration color: Reduction co	olor:
DETERMINATION OF HIGH GROUNDWATER ELEVATION	
Observed depth to stabilized phreatic water: inches below grade	
Observed water weeping from side of deep hole: inches below grade	
Observed depth to redoximorphic features: inches below grade	
DEPTH OF NATURALLY OCCURRING PERVIOUS MATERIAL: ► 7.66 feet	
Donth of naturally accurring particular material in TD16.2 Upper boundary: 10"	
Depth of naturally occurring pervious material in TP16-3 Upper boundary: <u>10"</u> Lower boundary: <u>102"</u>	
Certification	
Leastifished Least summation and the the Demotion of Africa in a sector biogenerated Device strengther and the 210 CMD 15 017 to a	J
I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to c evaluations and that the above analysis has been performed by me consistent with the required training, expertise and e	xperience described in
310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation For with 310 CMR 15.017.	orm, are accurate and in accordance
Alexander F. Parker Certified Soil Evaluator #1848	une 1998
Printed name of evaluator & certification number D	ate of Soil Evaluator Certification
Mr. John Coulon, Topsfield Director of Public Health	07/06/16
	Date of soil testing
	and of som testing

TP16-4 DEEP OBSERVATION HOLE

470 Boston Street, Topsfield, Massachusetts

Date:Wednesday, July 06, 2016Time:08:15Weather:Sunny, dry, ~85°F, Northeast breezePosition on landscape:BackslopeSlope aspect:NorthwesterlyLand Cover:Meadow grassProperty line:10⁺ feetDrainage way:50⁺ feetDrinking water well:100⁺ feetWetlands:100⁺ feetIpswich River:400⁺ feetOpen water body:400⁺ feetAbutting septic system:NA

SOIL PROFILE ► TP16-4

Depth below land surface (inches)	Soil Horizon/ Layer	Soil Texture (USDA/ NRCS)	Soil Color (Munsell)	Redoxomorphic Features/ ESHGWT	Consistence, grade, size, structure, grain size, soil moisture state, roots, horizon boundary, clasts, stratification, artifacts, restrictive features, etc.
00 → 10"	A _P	Sandy Loam	10YR 3/2 very dark grayish brown	none observed	Very friable; moderate-grade fine to medium subangular granular structure; weak cohesive matrix; fine grained mineral content; slightly damp; common fine to medium roots; free of clasts; clear wavy boundary.
10 → 16"	B_{W}	Sandy Loam	2.5Y 6/6 olive yellow	none observed	Very friable; weak-grade fine to medium angular blocky structure; weak cohesive matrix; gritty; mixed medium to mostly fine grained mineral content; damp; ~05% subrounded gravel content; diffuse wavy boundary.
16 → 97"	C ₁	Sand gravelly	10YR 4/4 dark yellowish brown	none observed	Loose; structurless; unstable; mixed fine to coarse grained sand; crudely stratified and well graded; ~10 -15% rounded to subrounded gravel content of mixed lithology; few strong red variegated iron stains on clasts and along bedding planes; stratified beds slightly dipping to the East; no refusal at test hole depth.
97 → 123"	C ₂	Sandy Loam gravelly	7.5YR 5/3 brown	none observed	Friable; moderate-grade medium-to-coarse subangular platy structure; mixed fine to coarse grained mineral content; well- graded; moderately compact matrix; crudely stratified with minor imbrication of clasts; somewhat silty; damp; ~15-20% scattered subangular-to-subrounded gravel and ~10% scattered subangular- cobble content of mixed lithology; clasts somewhat tightly nested in matrix; compactness slightly increases with depth; no bedrock contact at test hole depth and no apparent water observed.

 Depth to bedrock:
 >123"
 Seasonal High Groundwater Table:
 >123"
 Apparent water table:
 >123"

 420B - Canton fine sandy loam (coarse loamy over sandy, mixed, mesic, Typic Dystrochrepts), 03-08% slopes

TP16-4 DEEP OBSERVATION HOLE

470 Boston Street, Topsfield, Massachusetts

DEPTH TO APPARENT/ PHREATIC GROUNDWATER TABLE: None Observed
Apparent water seeping from pit face: (below land surface) Depth to stabilized apparent water: (below land surface)
Soil moisture state: Damp
ESTIMATED SEASONAL HIGH GROUNDWATER TABLE: None Observed
Depth of Estimated Seasonal High Groundwater Table: (below land surface)
Type: Size: Contrast:
Shape: Moisture state:
Hardness: Boundary: Concentration color: Reduction color:
DETERMINATION OF HIGH GROUNDWATER ELEVATION
Observed depth to stabilized phreatic water: inches below grade
Observed water weeping from side of deep hole: inches below grade
Observed depth to redoximorphic features: inches below grade
DEPTH OF NATURALLY OCCURRING PERVIOUS MATERIAL: ► 9.42 feet
Depth of naturally occurring pervious material in TP16-4 Upper boundary: <u>10</u> "
Lower boundary: <u>123</u> "
Certification
I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct
evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in
310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.017.
Alexander F. Parker Certified Soil Evaluator #1848 June 1998
Printed name of evaluator & certification number Date of Soil Evaluator Certification
Mr. John Coulon, Topsfield Director of Public Health 07/06/16
Town of Topsfield witness Date of soil testing

9

TP16-5 DEEP OBSERVATION HOLE

470 Boston Street, Topsfield, Massachusetts

Date: Wednesday, July	06, 2016	Time: <u>09:47</u>	Weather: Su	<u>ınny, dry, ~85°F, N</u>	Northeast breeze
Position on landscape: I	Backslope	Slope aspect: <u>N</u>	Northwesterly	Land Cover: M	eadow grass
Property line: <u>10⁺ feet</u>	Drainage	way: <u>50⁺ feet</u>	Drinking wat	er well: 100^+ feet	
Wetlands: <u>100⁺ feet</u>	Ipswich R	iver: <u>400⁺ feet</u>	Open water b	ody: <u>400⁺ feet</u>	Abutting septic system: <u>NA</u>

SOIL PROFILE ► TP16-5

Depth below land surface (inches)	Soil Horizon/ Layer	Soil Texture (USDA/ NRCS)	Soil Color (Munsell)	Redoxomorphic Features/ ESHGWT	Consistence, grade, size, structure, grain size, soil moisture state, roots, horizon boundary, clasts, stratification, artifacts, restrictive features, etc.
00 → 11"	A _P	Sandy Loam	10YR 3/2 very dark grayish brown	none observed	Very friable; moderate-grade fine to medium subangular granular structure; weak cohesive matrix; fine grained mineral content; slightly damp; common fine to medium roots; free of clasts; clear wavy boundary.
11 → 16"	B_{W}	Sandy Loam	2.5Y 6/6 olive yellow	none observed	Very friable; weak-grade fine to medium angular blocky structure; weak cohesive matrix; gritty; mixed medium to mostly fine grained mineral content; damp; ~05% subrounded gravel content; diffuse wavy boundary.
16 → 121"	С	Sandy Loam gravelly	2.5Y 5/4 lite olive brown	119" (f,2,d) 7.5YR5/8 10Y7/1	Friable; moderate-grade medium-to-coarse subangular platy structure; mixed fine to coarse grained mineral content; well- graded; moderately compact matrix; crudely stratified with minor imbrication of clasts; somewhat silty; damp; ~15-20% scattered subangular-to-subrounded gravel and ~15% scattered subangular- cobble content of mixed lithology; clasts somewhat tightly nested in matrix; compactness slightly increases with depth; no bedrock contact at test hole depth and no apparent water observed.

 Depth to bedrock: >121"
 Apparent water table: >121"

 420B - Canton fine sandy loam (coarse loamy over sandy, mixed, mesic, Typic Dystrochrepts), 03-08% slopes

TP16-5 DEEP OBSERVATION HOLE

470 Boston Street, Topsfield, Massachusetts

DEPTH TO APPARENT/ PHREATIC GROUNDWATER TABLE: None Observed
Apparent water seeping from pit face: (below land surface) Depth to stabilized apparent water: (below land surface)
Soil moisture state: Damp
ESTIMATED SEASONAL HIGH GROUNDWATER TABLE:
Depth of Estimated Seasonal High Groundwater Table: 119" (below land surface)
Type: Masses on and within blocky peds Abundance: Few Size: Medium Contrast: Distinct
Shape: Irregular spheroidal Moisture state: Damp Location: C matrix

Hardness: Soft Boundary: Diffuse Concentration color: 7.5YR 5/8 (red) Redu

Reduction color: <u>10Y 7/1 (bluish gray)</u>

DETERMINATION OF HIGH GROUNDWATER ELEVATION

Observed depth to stabilized phreatic water:		inches below grade
Observed water weeping from side of deep hole:		inches below grade
Observed depth to redoximorphic features:	<u>119"</u>	inches below grade

DEPTH OF NATURALLY OCCURRING PERVIOUS MATERIAL: ► 9.16 feet

Depth of naturally occurring pervious material in TP16-5

Upper boundary: <u>11"</u> Lower boundary: <u>121"</u>

Certification

I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.017.

Alexander F. Parker Certified Soil Evaluator #1848

Printed name of evaluator & certification number

Mr. John Coulon, Topsfield Director of Public Health

Town of Topsfield witness

June 1998

Date of Soil Evaluator Certification

07/06/16 Date of soil testing

TP16-6 DEEP OBSERVATION HOLE

470 Boston Street, Topsfield, Massachusetts

Date: Wednesday, July 06, 2	2016 Time: <u>09:54</u>	Weather: <u>Sunny, dry, ~85°F, N</u>	lortheast breeze
Position on landscape: Back	slope Slope aspect: <u>No</u>	orthwesterly Land Cover: M	eadow grass
Property line: 10^+ feet I	Drainage way: 50 ⁺ feet	Drinking water well: 100^+ feet	
Wetlands: 100^+ feet Ip	swich River: <u>400⁺ feet</u>	Open water body: 400^+ feet	Abutting septic system: <u>NA</u>

SOIL PROFILE ► TP16-6

Depth below land surface (inches)	Soil Horizon/ Layer	Soil Texture (USDA/ NRCS)	Soil Color (Munsell)	Redoxomorphic Features/ ESHGWT	Consistence, grade, size, structure, grain size, soil moisture state, roots, horizon boundary, clasts, stratification, artifacts, restrictive features, etc.
00 → 12"	A _P	Sandy Loam	10YR 3/2 very dark grayish brown	none observed	Very friable; moderate-grade fine to medium subangular granular structure; weak cohesive matrix; fine grained mineral content; slightly damp; common fine to medium roots; free of clasts; clear wavy boundary.
11 → 25"	B_{W}	Sandy Loam	2.5Y 6/6 olive yellow	none observed	Very friable; weak-grade fine to medium angular blocky structure; weak cohesive matrix; gritty; mixed medium to mostly fine grained mineral content; damp; ~05% subrounded gravel content; diffuse wavy boundary.
16 → 137" [*]	С	Sandy Loam gravelly	2.5Y 5/4 lite olive brown	120" (f,2,d) 7.5YR5/8 10Y7/1	Friable; moderate-grade medium-to-coarse subangular platy structure; mixed fine to coarse grained mineral content; well- graded; moderately compact matrix; crudely stratified with minor imbrication of clasts; somewhat silty; damp; ~15-20% scattered subangular-to-subrounded gravel and ~15% scattered subangular- cobble content of mixed lithology; clasts somewhat tightly nested in matrix; compactness slightly increases with depth; no bedrock contact at test hole depth and no apparent water observed.

 Depth to bedrock: <a>137"
 Seasonal High Groundwater Table: <a>120"
 Apparent water table: <a>137"

 420B - Canton fine sandy loam (coarse loamy over sandy, mixed, mesic, Typic Dystrochrepts), 03-08% slopes

TP16-6 DEEP OBSERVATION HOLE

470 Boston Street, Topsfield, Massachusetts

 DEPTH TO APPARENT/ PHREATIC GROUNDWATER TABLE:
 None Observed

 Apparent water seeping from pit face:
 (below land surface)
 Depth to stabilized apparent water:
 (below land surface)

 Soil moisture state:
 Damp

 ESTIMATED SEASONAL HIGH GROUNDWATER TABLE:

 Depth of Estimated Seasonal High Groundwater Table:
 120" (below land surface)

 Type:
 Masses on and within blocky peds
 Abundance: Few
 Size: Medium
 Contrast: Distinct

Shape: Irregular spheroidal Moisture state: Damp Location: C matrix

Hardness: Soft Boundary: Diffuse Concentration color: 7.5YR 5/8 (red) Reduction color: 10Y 7/1 (bluish gray)

DETERMINATION OF HIGH GROUNDWATER ELEVATION

Observed depth to stabilized phreatic water:		inches below grade
Observed water weeping from side of deep hole:		inches below grade
Observed depth to redoximorphic features:	<u>120"</u>	inches below grade

DEPTH OF NATURALLY OCCURRING PERVIOUS MATERIAL: ► 10.41 feet

Depth of naturally occurring pervious material in TP16-6

Upper boundary: <u>12</u>" Lower boundary: <u>137</u>"

Certification

I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.017.

Alexander F. Parker Certified Soil Evaluator #1848

Printed name of evaluator & certification number

Mr. John Coulon, Topsfield Director of Public Health

Town of Topsfield witness

June 1998

Date of Soil Evaluator Certification

07/06/16

Date of soil testing

TP16-7 DEEP OBSERVATION HOLE

470 Boston Street, Topsfield, Massachusetts

Date: Wednesday, July 0	06, 2016 Time: <u>10:15</u>	Weather: Sunny, dry, ~85°F, N	lortheast breeze
Position on landscape: Ba	ackslope Slope aspect: <u>N</u>	Northwesterly Land Cover: M	eadow grass
Property line: <u>10⁺ feet</u>	Drainage way: 50 ⁺ feet	Drinking water well: <u>100⁺ feet</u>	
Wetlands: <u>100⁺ feet</u>	Ipswich River: <u>400⁺ feet</u>	Open water body: <u>400⁺ feet</u>	Abutting septic system: <u>NA</u>

SOIL PROFILE ► TP16-7

Depth below land surface (inches)	Soil Horizon/ Layer	Soil Texture (USDA/ NRCS)	Soil Color (Munsell)	Redoxomorphic Features/ ESHGWT	Consistence, grade, size, structure, grain size, soil moisture state, roots, horizon boundary, clasts, stratification, artifacts, restrictive features, etc.
00 → 13"	A _P	Sandy Loam	10YR 3/2 very dark grayish brown	none observed	Very friable; moderate-grade fine to medium subangular granular structure; weak cohesive matrix; fine grained mineral content; slightly damp; common fine to medium roots; free of clasts; clear wavy boundary.
13 → 38"	B_{W}	Sandy Loam	2.5Y 6/6 olive yellow	none observed	Very friable; weak-grade fine to medium angular blocky structure; weak cohesive matrix; gritty; mixed medium to mostly fine grained mineral content; damp; ~05% subrounded gravel content; diffuse wavy boundary.
38 → 130"	С	Sandy Loam gravelly	2.5Y 5/4 lite olive brown	120" (f,2,d) 7.5YR5/8 10Y7/1	Friable; moderate-grade medium-to-coarse subangular platy structure; mixed fine to coarse grained mineral content; well- graded; moderately compact matrix; crudely stratified with minor imbrication of clasts; somewhat silty; damp; ~15-20% scattered subangular-to-subrounded gravel and ~15% scattered subangular- cobble content of mixed lithology; clasts somewhat tightly nested in matrix; compactness slightly increases with depth; no bedrock contact at test hole depth and no apparent water observed.

 Depth to bedrock: <a>130"
 Seasonal High Groundwater Table: <a>120"
 Apparent water table: <a>130"

 420B - Canton fine sandy loam (coarse loamy over sandy, mixed, mesic, Typic Dystrochrepts), 03-08% slopes

TP16-7 DEEP OBSERVATION HOLE

470 Boston Street, Topsfield, Massachusetts

DEPTH TO APPARENT/ PHREATIC GROUNDWATER TABLE: None Observed
Apparent water seeping from pit face: (below land surface) Depth to stabilized apparent water: (below land surface)
Soil moisture state: <u>Damp</u>
ESTIMATED SEASONAL HIGH GROUNDWATER TABLE:
Depth of Estimated Seasonal High Groundwater Table: <u>120</u> " (below land surface)

Type: Masses on and within blocky pedsAbundance: FewSize: MediumContrast: DistinctShape: Irregular spheroidalMoisture state: DampLocation: C matrixHardness: SoftBoundary: DiffuseConcentration color: 7.5YR 5/8 (red)Reduction color: 10Y 7/1 (bluish gray)

DETERMINATION OF HIGH GROUNDWATER ELEVATION

Observed depth to stabilized phreatic water:		inches below grade
Observed water weeping from side of deep hole:		inches below grade
Observed depth to redoximorphic features:	<u>120"</u>	inches below grade

DEPTH OF NATURALLY OCCURRING PERVIOUS MATERIAL: ► 10.41 feet

Depth of naturally occurring pervious material in TP16-7

Upper boundary: <u>12</u>" Lower boundary: <u>137</u>"

Certification

I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.017.

Alexander F. Parker Certified Soil Evaluator #1848

Printed name of evaluator & certification number

Mr. John Coulon, Topsfield Director of Public Health

Town of Topsfield witness

June 1998

Date of Soil Evaluator Certification

07/06/16

Date of soil testing

TP16-8 DEEP OBSERVATION HOLE

470 Boston Street, Topsfield, Massachusetts

Date: Wednesday, July	06, 2016	Time: <u>10:25</u>	Weather: Su	<u>inny, dry, ~85°F, N</u>	Jortheast breeze
Position on landscape: <u>H</u>	<u> Backslope</u>	Slope aspect: <u>N</u>	orthwesterly	Land Cover: M	eadow grass
Property line: <u>10⁺ feet</u>	Drainage	e way: <u>50⁺ feet</u>	Drinking wate	er well: 100^+ feet	
Wetlands: <u>100⁺ feet</u>	Ipswich R	iver: <u>400⁺ feet</u>	Open water b	ody: <u>400⁺ feet</u>	Abutting septic system: <u>NA</u>

SOIL PROFILE ► TP16-8

Depth below land surface (inches)	Soil Horizon/ Layer	Soil Texture (USDA/ NRCS)	Soil Color (Munsell)	Redoxomorphic Features/ ESHGWT	Consistence, grade, size, structure, grain size, soil moisture state, roots, horizon boundary, clasts, stratification, artifacts, restrictive features, etc.
00 → 12"	A _P	Sandy Loam	10YR 3/2 very dark grayish brown	none observed	Very friable; moderate-grade fine to medium subangular granular structure; weak cohesive matrix; fine grained mineral content; slightly damp; common fine to medium roots; free of clasts; clear wavy boundary.
12 → 23"	B_{W}	Sandy Loam	2.5Y 6/6 olive yellow	none observed	Very friable; weak-grade fine to medium angular blocky structure; weak cohesive matrix; gritty; mixed medium to mostly fine grained mineral content; damp; ~05% subrounded gravel content; diffuse wavy boundary.
23 → 133"	С	Sandy Loam gravelly	2.5Y 5/4 lite olive brown	130" (f,2,d) 7.5YR5/8 10Y7/1	Friable; moderate-grade medium-to-coarse subangular platy structure; mixed fine to coarse grained mineral content; well- graded; moderately compact matrix; crudely stratified with minor imbrication of clasts; somewhat silty; damp; ~15-20% scattered subangular-to-subrounded gravel and ~15% scattered subangular- cobble content of mixed lithology; clasts somewhat tightly nested in matrix; compactness slightly increases with depth; no bedrock contact at test hole depth and no apparent water observed.

 Depth to bedrock:
 >133"
 Seasonal High Groundwater Table:
 130"
 Apparent water table:
 >133"

 420B - Canton fine sandy loam (coarse loamy over sandy, mixed, mesic, Typic Dystrochrepts), 03-08% slopes

TP16-8 DEEP OBSERVATION HOLE

470 Boston Street, Topsfield, Massachusetts

 DEPTH TO APPARENT/ PHREATIC GROUNDWATER TABLE:
 None Observed

 Apparent water seeping from pit face:
 (below land surface)
 Depth to stabilized apparent water:
 (below land surface)

 Soil moisture state:
 Damp

 ESTIMATED SEASONAL HIGH GROUNDWATER TABLE:
 Depth of Estimated Seasonal High Groundwater Table:
 130" (below land surface)

Type: Masses on and within blocky pedsAbundance: FewSize: MediumContrast: DistinctShape: Irregular spheroidalMoisture state: DampLocation: C matrixHardness: SoftBoundary: DiffuseConcentration color: 7.5YR 5/8 (red)Reduction color: 10Y 7/1 (bluish gray)

DETERMINATION OF HIGH GROUNDWATER ELEVATION

Observed depth to stabilized phreatic water:		inches below grade
Observed water weeping from side of deep hole:		inches below grade
Observed depth to redoximorphic features:	<u>130"</u>	inches below grade

DEPTH OF NATURALLY OCCURRING PERVIOUS MATERIAL: ► 10.08 feet

Depth of naturally occurring pervious material in TP16-8 Upper

Upper boundary: <u>12"</u> Lower boundary: <u>133"</u>

Certification

I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.017.

Alexander F. Parker Certified Soil Evaluator #1848

Printed name of evaluator & certification number

Mr. John Coulon, Topsfield Director of Public Health

Town of Topsfield witness

June 1998

Date of Soil Evaluator Certification

07/06/16

Date of soil testing

TP16-9 DEEP OBSERVATION HOLE

470 Boston Street, Topsfield, Massachusetts

Date:Wednesday, July 06, 2016Time:11:15Weather:Sunny, dry, ~85°F, Northeast breezePosition on landscape:BackslopeSlope aspect:NorthwesterlyLand Cover:Lightly woodedProperty line:10⁺ feetDrainage way:50⁺ feetDrinking water well:100⁺ feetWetlands:100⁺ feetIpswich River:400⁺ feetOpen water body:400⁺ feetAbutting septic system:

SOIL PROFILE ► TP16-9

Depth below land surface (inches)	Soil Horizon/ Layer	Soil Texture (USDA/ NRCS)	Soil Color (Munsell)	Redoxomorphic Features/ ESHGWT	Consistence, grade, size, structure, grain size, soil moisture state, roots, horizon boundary, clasts, stratification, artifacts, restrictive features, etc.
00 → 07"	A	Sandy Loam	10YR 3/2 very dark grayish brown	none observed	Very friable; moderate-grade fine to medium subangular granular structure; weak cohesive matrix; fine grained mineral content; slightly damp; common fine to medium roots; free of clasts; clear wavy boundary.
07 → 16"	B_{W}	Sandy Loam	2.5Y 6/6 olive yellow	none observed	Very friable; weak-grade fine to medium angular blocky structure; weak cohesive matrix; gritty; mixed medium to mostly fine grained mineral content; damp; ~05% subrounded gravel content; diffuse wavy boundary.
16 → 90"	C ₁	Sand gravelly	10YR 4/4 dark yellowish brown	none observed	Loose; structurless; unstable; mixed fine to coarse grained sand; crudely stratified and well graded; ~10 -15% rounded to subrounded gravel content of mixed lithology; few strong red variegated iron stains on clasts and along bedding planes; stratified beds slightly dipping to the East; no refusal at test hole depth.
90 → 115"	C ₂	Sandy Loam gravelly	7.5YR 5/3 brown	93" (c,3,p) 7.5YR5/8 10Y7/1	Friable; moderate-grade medium-to-coarse subangular platy structure; mixed fine to coarse grained mineral content; well- graded; moderately compact matrix; crudely stratified with minor imbrication of clasts; somewhat silty; damp; ~20-25% scattered subangular-to-subrounded gravel and ~10% scattered subangular- cobble content of mixed lithology; clasts somewhat tightly nested in matrix; compactness slightly increases with depth; no bedrock contact at test hole depth and apparent water observed at 135".

TP16-9 DEEP OBSERVATION HOLE

470 Boston Street, Topsfield, Massachusetts

DEPTH TO APPARENT/ PHREATIC GROUNDWATER TABLE:

Apparent water seeping from pit face: <u>135</u>" (below land surface) Depth to stabilized apparent water: <u>135</u>" (below land surface) Soil moisture state: <u>Damp to wet</u>

ESTIMATED SEASONAL HIGH GROUNDWATER TABLE:

Depth of Estimated Seasonal High Groundwater Table: 93" (below land surface)							
Type: <u>Masses on and within blocky peds</u> Abundance: <u>Common</u> Size: <u>Coarse</u> Contrast: <u>Prominent</u>							
Shape: Irregular spheroidal Moisture	state: <u>Damp</u> Location: <u>C matrix</u>						
Hardness: <u>Soft</u> Boundary: <u>Diffuse</u>	Concentration color: 7.5YR 5/8 (red)	Reduction color: 10Y 7/1 (bluish gray)					

DETERMINATION OF HIGH GROUNDWATER ELEVATION

Observed depth to stabilized phreatic water:	135"	inches below grade
Observed water weeping from side of deep hole:	<u>135"</u>	inches below grade
Observed depth to redoximorphic features:	93"	inches below grade

DEPTH OF NATURALLY OCCURRING PERVIOUS MATERIAL: ► 8.75 feet

Depth of naturally occurring pervious material in TP16-9

Upper boundary: <u>14"</u> Lower boundary: <u>119"</u>

Certification

I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.017.

Alexander F. Parker Certified Soil Evaluator #1848

Printed name of evaluator & certification number

Mr. John Coulon, Topsfield Director of Public Health

Town of Topsfield witness

June 1998 Date of Soil Evaluator Certification

07/06/16

Date of soil testing

TP16-10 DEEP OBSERVATION HOLE

470 Boston Street, Topsfield, Massachusetts

Date: Wednesday, July	06, 2016	Time: <u>11:32</u>	Weather: Su	<u>nny, dry, ~85°F, N</u>	Jortheast breeze
Position on landscape: <u>H</u>	<u> Backslope</u>	Slope aspect: <u>N</u>	orthwesterly	Land Cover: <u>Li</u>	ghtly wooded
Property line: <u>10⁺ feet</u>	Drainage	e way: <u>50⁺ feet</u>	Drinking wat	er well: 100^+ feet	
Wetlands: <u>100⁺ feet</u>	Ipswich R	iver: <u>400⁺ feet</u>	Open water b	ody: <u>400⁺ feet</u>	Abutting septic system: <u>NA</u>

SOIL PROFILE ► TP16-10

Depth below land surface (inches)	Soil Horizon/ Layer	Soil Texture (USDA/ NRCS)	Soil Color (Munsell)	Redoxomorphic Features/ ESHGWT	Consistence, grade, size, structure, grain size, soil moisture state, roots, horizon boundary, clasts, stratification, artifacts, restrictive features, etc.
00 → 12"	A	Sandy Loam	10YR 3/2 very dark grayish brown	none observed	Very friable; moderate-grade fine to medium subangular granular structure; weak cohesive matrix; fine grained mineral content; slightly damp; common fine to medium roots; free of clasts; clear wavy boundary.
12 → 18"	B_{W}	Sandy Loam	2.5Y 6/6 olive yellow	none observed	Very friable; weak-grade fine to medium angular blocky structure; weak cohesive matrix; gritty; mixed medium to mostly fine grained mineral content; damp; ~05% subrounded gravel content; diffuse wavy boundary.
18 → 124"	С	Sandy Loam gravelly	2.5Y 5/4 lite olive brown	90" (f,2,d) 7.5YR5/8 10Y7/1	Friable; moderate-grade medium-to-coarse subangular platy structure; mixed fine to coarse grained mineral content; well- graded; moderately compact matrix; crudely stratified with minor imbrication of clasts; somewhat silty; damp; ~15-20% scattered subangular-to-subrounded gravel and ~15% scattered subangular- cobble content of mixed lithology; clasts somewhat tightly nested in matrix; compactness slightly increases with depth; no bedrock contact at test hole depth and no apparent water observed.

 Depth to bedrock:
 >124"
 Seasonal High Groundwater Table:
 90"
 Apparent water table:
 >124"

 420B - Canton fine sandy loam (coarse loamy over sandy, mixed, mesic, Typic Dystrochrepts), 03-08% slopes

TP16-10 DEEP OBSERVATION HOLE

470 Boston Street, Topsfield, Massachusetts

 DEPTH TO APPARENT/ PHREATIC GROUNDWATER TABLE:
 None Observed

 Apparent water seeping from pit face:
 (below land surface)
 Depth to stabilized apparent water:
 (below land surface)

 Soil moisture state:
 Damp

ESTIMATED SEASONAL HIGH GROUNDWATER TABLE:

Depth of Estimated Seasonal High Groundwater Table: 90" (below land surface)Type: Masses on and within blocky pedsAbundance: FewSize: MediumContrast: DistinctShape: Irregular spheroidalMoisture state: DampLocation: C matrixHardness: SoftBoundary: DiffuseConcentration color: 7.5YR 5/8 (red)Reduction color: 10Y 7/1 (bluish gray)

DETERMINATION OF HIGH GROUNDWATER ELEVATION

Observed depth to stabilized phreatic water:		inches below grade
Observed water weeping from side of deep hole:		inches below grade
Observed depth to redoximorphic features:	90"	inches below grade

DEPTH OF NATURALLY OCCURRING PERVIOUS MATERIAL: ► 9.33 feet

Depth of naturally occurring pervious material in TP16-10 Upper b

Upper boundary: <u>12"</u> Lower boundary: <u>124"</u>

Certification

I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.017.

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Printed name of evaluator & certification number

Mr. John Coulon, Topsfield Director of Public Health

Town of Topsfield witness

June 1998

Date of Soil Evaluator Certification

07/06/16

Date of soil testing

TP16-11 DEEP OBSERVATION HOLE

470 Boston Street, Topsfield, Massachusetts

Date: Wednesday, July 06, 2	2016 Time: <u>11:40</u>	Weather: Sunny, dry, ~85°F, N	ortheast breeze
Position on landscape: Backs	slope Slope aspect: Nor	thwesterly Land Cover: Li	ghtly wooded
Property line: 10^+ feet D	rainage way: 50 ⁺ feet	Drinking water well: 100^+ feet	
Wetlands: 100^+ feet Ips	wich River: <u>400⁺ feet</u>	Open water body: 400^+ feet	Abutting septic system: <u>NA</u>

SOIL PROFILE ► TP16-11

Depth below land surface (inches)	Soil Horizon/ Layer	Soil Texture (USDA/ NRCS)	Soil Color (Munsell)	Redoxomorphic Features/ ESHGWT	Consistence, grade, size, structure, grain size, soil moisture state, roots, horizon boundary, clasts, stratification, artifacts, restrictive features, etc.
00 → 08"	A	Sandy Loam	10YR 3/2 very dark grayish brown	none observed	Very friable; moderate-grade fine to medium subangular granular structure; weak cohesive matrix; fine grained mineral content; slightly damp; common fine to medium roots; free of clasts; clear wavy boundary.
08 → 21"	B_{W}	Sandy Loam	2.5Y 6/6 olive yellow	none observed	Very friable; weak-grade fine to medium angular blocky structure; weak cohesive matrix; gritty; mixed medium to mostly fine grained mineral content; damp; ~05% subrounded gravel content; diffuse wavy boundary.
21 → 143	С	Sandy Loam gravelly	2.5Y 5/4 lite olive brown	96" (f,2,d) 7.5YR5/8 10Y7/1	Friable; moderate-grade medium-to-coarse subangular platy structure; mixed fine to coarse grained mineral content; well- graded; moderately compact matrix; crudely stratified with minor imbrication of clasts; somewhat silty; damp; ~15-20% scattered subangular-to-subrounded gravel and ~15% scattered subangular- cobble content of mixed lithology; clasts somewhat tightly nested in matrix; compactness slightly increases with depth; no bedrock contact at test hole depth and no apparent water observed.

Depth to bedrock: <a>143"Seasonal High Groundwater Table: <a>96"Apparent water table: <a>143"420B - Canton fine sandy loam (coarse loamy over sandy, mixed, mesic, Typic Dystrochrepts), 03-08% slopes

TP16-11 DEEP OBSERVATION HOLE

470 Boston Street, Topsfield, Massachusetts

 DEPTH TO APPARENT/ PHREATIC GROUNDWATER TABLE:
 None Observed

 Apparent water seeping from pit face:
 (below land surface)
 Depth to stabilized apparent water:
 (below land surface)

 Soil moisture state:
 Damp

ESTIMATED SEASONAL HIGH GROUNDWATER TABLE:

Depth of Estimated Seasonal High Groundwater Table: 96" (below land surface)Type: Masses on and within blocky pedsAbundance: FewSize: MediumContrast: DistinctShape: Irregular spheroidalMoisture state: DampLocation: C matrixHardness: SoftBoundary: DiffuseConcentration color: 7.5YR 5/8 (red)Reduction color: 10Y 7/1 (bluish gray)

DETERMINATION OF HIGH GROUNDWATER ELEVATION

Observed depth to stabilized phreatic water:		inches below grade
Observed water weeping from side of deep hole:		inches below grade
Observed depth to redoximorphic features:	<u>96"</u>	inches below grade

DEPTH OF NATURALLY OCCURRING PERVIOUS MATERIAL: ► 9.33 feet

Depth of naturally occurring pervious material in TP16-11 Upper

Upper boundary: <u>12"</u> Lower boundary: <u>124"</u>

Certification

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Printed name of evaluator & certification number

Mr. John Coulon, Topsfield Director of Public Health

Town of Topsfield witness

June 1998

Date of Soil Evaluator Certification

07/06/16

Date of soil testing

TP16-12 DEEP OBSERVATION HOLE

470 Boston Street, Topsfield, Massachusetts

Date: Wednesday, July 06,	<u>2016</u> Time: <u>11:59</u>	Weather: <u>Sunny, dry, ~85°F, N</u>	lortheast breeze
Position on landscape: Back	kslope Slope aspect: <u>No</u>	orthwesterly Land Cover: Li	ghtly wooded
Property line: 10^+ feet I	Drainage way: 50 ⁺ feet	Drinking water well: <u>100⁺ feet</u>	
Wetlands: 100^+ feet Ip	oswich River: <u>400⁺ feet</u>	Open water body: 400^+ feet	Abutting septic system: <u>NA</u>

SOIL PROFILE ► TP16-12

Depth below land surface (inches)	Soil Horizon/ Layer	Soil Texture (USDA/ NRCS)	Soil Color (Munsell)	Redoxomorphic Features/ ESHGWT	Consistence, grade, size, structure, grain size, soil moisture state, roots, horizon boundary, clasts, stratification, artifacts, restrictive features, etc.
00 → 12"	A	Sandy Loam	10YR 3/2 very dark grayish brown	none observed	Very friable; moderate-grade fine to medium subangular granular structure; weak cohesive matrix; fine grained mineral content; slightly damp; common fine to medium roots; free of clasts; clear wavy boundary.
12 → 28"	B_{W}	Sandy Loam	2.5Y 6/6 olive yellow	none observed	Very friable; weak-grade fine to medium angular blocky structure; weak cohesive matrix; gritty; mixed medium to mostly fine grained mineral content; damp; ~05% subrounded gravel content; diffuse wavy boundary.
28 → 123	С	Sandy Loam gravelly	2.5Y 5/4 lite olive brown	94" (f,2,d) 7.5YR5/8 10Y7/1	Friable; moderate-grade medium-to-coarse subangular platy structure; mixed fine to coarse grained mineral content; well- graded; moderately compact matrix; crudely stratified with minor imbrication of clasts; somewhat silty; damp; ~15-20% scattered subangular-to-subrounded gravel and ~15% scattered subangular- cobble content of mixed lithology; clasts somewhat tightly nested in matrix; compactness slightly increases with depth; no bedrock contact at test hole depth and no apparent water observed.

 Depth to bedrock:
 >123"
 Seasonal High Groundwater Table:
 94"
 Apparent water table:
 >123"

 420B - Canton fine sandy loam (coarse loamy over sandy, mixed, mesic, Typic Dystrochrepts), 03-08% slopes

TP16-12 DEEP OBSERVATION HOLE

470 Boston Street, Topsfield, Massachusetts

 DEPTH TO APPARENT/ PHREATIC GROUNDWATER TABLE:
 None Observed

 Apparent water seeping from pit face:
 (below land surface)
 Depth to stabilized apparent water:
 (below land surface)

 Soil moisture state:
 Damp

ESTIMATED SEASONAL HIGH GROUNDWATER TABLE:

Depth of Estimated Seasonal High Groundwater Table: <u>94</u>" (below land surface) Type: <u>Masses on and within blocky peds</u> Abundance: <u>Few</u> Size: <u>Medium</u> Contrast: <u>Distinct</u> Shape: <u>Irregular spheroidal</u> Moisture state: <u>Damp</u> Location: <u>C matrix</u> Hardness: <u>Soft</u> Boundary: <u>Diffuse</u> Concentration color: <u>7.5YR 5/8 (red)</u> Reduction color: <u>10Y 7/1 (bluish gray)</u>

DETERMINATION OF HIGH GROUNDWATER ELEVATION

Observed depth to stabilized phreatic water:		inches below grade
Observed water weeping from side of deep hole:	. <u></u>	inches below grade
Observed depth to redoximorphic features:	94"	inches below grade

DEPTH OF NATURALLY OCCURRING PERVIOUS MATERIAL: ► 9.25 feet

Depth of naturally occurring pervious material in TP16-12 Upper boundary: <u>12</u>" Lower boundary: <u>12</u>"

Certification

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Mr. John Coulon, Topsfield Director of Public Health

Town of Topsfield witness

June 1998

Date of Soil Evaluator Certification

07/06/16

Date of soil testing

SOIL SUITABILITY PERCOLATION TEST COMMONWEALTH OF MASSACHUSETTS TOPSFIELD, MASSACHUSETTS

470 Boston Street, Topsfield, Massachusetts

Percolation Test	Percolation Test-1 TP16-2	Percolation Test-2 TP16-4		
Depth of test:	Depth to shelf: 48" 66" Depth of hole: 18"	Depth to shelf: 24" 42" Depth of hole: 18"		
Start presoak:	13:10	12:29		
End presoak:	13:25	12:44		
Time at 12" \rightarrow	13:25	12:44		
Time at $9" \rightarrow$	13:28	12:46		
Time at $6^{"} \rightarrow$	13:33	12:49		
Total time 9" to $6" \rightarrow$	5 minutes	3 minutes		
Rate (minutes per inch)	1.66 MPI	1.00 MPI		

Alexander F. Parker License #1848 Printed name of evaluator & license number $\frac{07/06/16}{\text{Date of percolation testing}}$

Mr. John Coulon, Topsfield Director of Public Health

Town of Topsfield witness

SOIL SUITABILITY PERCOLATION TEST COMMONWEALTH OF MASSACHUSETTS TOPSFIELD, MASSACHUSETTS

470 Boston Street, Topsfield, Massachusetts

Percolation Test	Percolation Test-3 TP16-8	Percolation Test-4 TP16-5		
Depth of test:	Depth to shelf: 45" 63" Depth of hole: 18"	Depth to shelf: 36" 54" Depth of hole: 18"		
Start presoak:	14:10	13:55		
End presoak:	14:25	12:44		
Time at 12" \rightarrow	14:25	12:44		
Time at 9" \rightarrow	14:30	12:46		
Time at 6" \rightarrow	14:44	12:49		
Total time 9" to $6" \rightarrow$	14 minutes	3 minutes		
Rate (minutes per inch)	4.66 MPI	1.00 MPI		

Alexander F. Parker License #1848 Printed name of evaluator & license number $\frac{07/06/16}{\text{Date of percolation testing}}$

Mr. John Coulon, Topsfield Director of Public Health

Town of Topsfield witness

SOIL SUITABILITY PERCOLATION TEST COMMONWEALTH OF MASSACHUSETTS TOPSFIELD, MASSACHUSETTS

470 Boston Street, Topsfield, Massachusetts

Percolation Test	Percolation Test-5 TP16-12	Percolation Test-6 TP16-10		
Depth of test:	Depth to shelf: 26" 54" Depth of hole: 18"	Depth to shelf: 36" 54" Depth of hole: 18"		
Start presoak:	15:03	15:55		
End presoak:	15:18	16:10		
Time at 12" \rightarrow	15:18	16:10		
Time at 9" \rightarrow	15:24	16:30		
Time at 6 " \rightarrow	15:31	16:52		
Total time 9" to $6" \rightarrow$	7 minutes	22 minutes		
Rate (minutes per inch)	2.33 MPI	7.33 MPI		

Alexander F. Parker License #1848 Printed name of evaluator & license number

Mr. John Coulon, Topsfield Director of Public Health

Town of Topsfield witness

 $\frac{07/06/16}{\text{Date of percolation testing}}$



Commonwealth of Massachusetts City/Town of Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

	Sarkis Development Company						
	Owner Name 470 Boston Street					Map 2, Lot 5	5
	Street Address					Map/Lot #	
	Topsfield			MA		01983	
	City			State		Zip Code	
B	Site Information						
1.	(Check one) X New Constr	ruction	Upgrade		Repair		
2.	Soil Survey Available?	x Yes	🗌 No	If yes:	UC Davis Web Soil S	urvey	420B, 421C
	Canton Fine Sandy Loam			Source Soil Map Unit Bedrock			
	Soil Name			Soil Limita			
	Sandy till			Morra	ine		
	Geologic/Parent Material			Landform			
3.	Surficial Geological Report Available?	🗌 Yes	X No	If yes:			
							Map Unit
4.	Flood Rate Insurance Map						
	Above the 500-year flood boundary? If Yes, continue to #5.	X Yes	🗌 No		e 100-year flood boundary A Zone A	/? X Yes	🗌 No
5.	Within a velocity zone?	Yes	X No				
6.	Within a Mapped Wetland Area?	X Yes	🗌 No	MassGI	S Wetland Data Layer: $^{ m W}$	Vooded Swamp Wetland Type	Deciduous/Mixe
7.	Current Water Resource Conditions	(USGS):	June, 2016 Month/Year	Range:	Above Normal	Normal X Bel	ow Normal
8.	Other references reviewed: N	J/A					



Commonwealth of Massachusetts City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C.	On-Site Revi	ew (minimum of tw	vo holes req	uired at every pro	oposed prim	ary and reserve dispos	al area)		
	Deep Observation Hole Number: TP-1			7/7/2016 8:00AM		Overcast, 65 deg	Overcast, 65 degrees		
		-		Date	Time	Weather			
1.	. Location								
	Ground Elevation at Surface of Hole: 74.0 feetDescription of Location:Crushed stone drive			Latit	ude/Longitude	42.664163 / -70.930328			
				ear end of paved d	riveway				
2.	. Land Use Open field				N/A		0-3%		
	(e.g., woodland, agricultural field, vacant lot, \vec{e} $Grass$			Morraine	Surface Stones	(e.g., cobbles, stones, boulders, et $\mathrm{N/A}$			
	Veg	etation		Landform Position on Landscape (SU, SH, BS, I			BS, FS, TS)		
3.	Distances from:	Open Water Body	N/A feet	_ Drainage Way	<u>N/A</u> feet	Wetlands	$\frac{200 + feet}{feet}$		
		Property Line	110' feet	_ Drinking Water	Well <u>N/A</u>	Other	N/A feet		
4.	Parent Material:	Sandy till		Unsuita	able Materials	Present: X Yes	□ No		
	If Yes: Dist	urbed Soil 🛛 🗌 F	ill Material	x Impervious Layer(s) 🛛 🖾 (/eathered/Fractured Rock	X Bedrock		
5	5. Groundwater Observed: Yes 🛛 Yo		x No	If yes:	N/A	N/A			
0.				ii yooi	-	ping from Pit Depth Sta	anding Water in Hole		
	Estimated Depth to	High Groundwater:	100"	65.7					
	inches			elevation	1				



Commonwealth of Massachusetts

City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (continued)

Deep Observation Hole Number:

TP-1

Depth (in.)	Soil Horizon/	l Horizon/ Soil Matrix: Color- Layer Moist (Munsell)	Red	loximorphic Feat	ures	Soil Texture	Coarse Fragments % by Volume		Soil Structure	Soil	Other
Depth (m.)	Layer		Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones		(Moist)	Other
2-0	Crushed stone										
0-66	2C1	5R3/6				SL	5%	10%			
66-108	2C2	5R3/6	100	7.5YR6/8	2%	SL	5%	15%			

Additional Notes:

Fractured/weathered rock throughout. No water noted, no weeping.

ESHWT @ 100" (2% mottles - concentrations)

Refusal @ 108" (Ledge)



Commonwealth of Massachusetts City/Town of Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

	Sarkis Development Company						
	Owner Name 470 Boston Street					Map 2, Lot 5	5
	Street Address					Map/Lot #	
	Topsfield			MA		01983	
	City			State		Zip Code	
B	Site Information						
1.	(Check one) X New Constr	ruction	Upgrade		Repair		
2.	Soil Survey Available?	x Yes	🗌 No	If yes:	UC Davis Web Soil S	urvey	420B, 421C
	Canton Fine Sandy Loam			Bedroc	Source k		Soil Map Unit
	Soil Name			Soil Limita			
	Sandy till			Morra	line		
	Geologic/Parent Material			Landform			
3.	Surficial Geological Report Available?	🗌 Yes	X No	If yes:			
							Map Unit
4.	Flood Rate Insurance Map						
	Above the 500-year flood boundary? If Yes, continue to #5.	X Yes	🗌 No		e 100-year flood boundary A Zone A	/? X Yes	🗌 No
5.	Within a velocity zone?	Yes	X No				
6.	Within a Mapped Wetland Area?	X Yes	🗌 No	MassGI	S Wetland Data Layer: $^{ m W}$	Vooded Swamp Wetland Type	Deciduous/Mixe
7.	Current Water Resource Conditions	(USGS):	June, 2016 Month/Year	Range:	Above Normal	Normal X Bel	ow Normal
8.	Other references reviewed: N	J/A					



Commonwealth of Massachusetts City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C.	On-Site Re	eview (minimum of t	wo holes req	uired at every pro	oposed prim	ary and reserve dispo	sal area)
	Deep Observat	tion Hole Number:	TP-2	7/7/2016	8:00AM	Overcast, 65 de	grees
		-		Date	Time	Weather	
1.	Location						
	Ground Elevation	on at Surface of Hole:	75.0	Latit	ude/Longitude	:42.664163 / -70.930328	3
	Description of L	ocation: Slope al		property line, 110'	from PL		
2.	Land Use	Open field			N/A		3-8%
		(e.g., woodland, agricultural fie Grass	ld, vacant lot, etc.)	Morraine	Surface Stones	(e.g., cobbles, stones, boulders, N/A	etc.) Slope (%)
		Vegetation		Landform		Position on Landscape (SU, SH,	BS, FS, TS)
3.	Distances from:	Open Water Body	N/A feet	_ Drainage Way	<u>N/A</u> feet	Wetlands	<u>300+ feet</u>
		Property Line	110' feet	Drinking Water	Well $\frac{N/A}{feet}$	Other	N/A feet
4.	Parent Material	Sandy till		Unsuit	able Materials	Present: X Yes	🗌 No
	If Yes:	Disturbed Soil	Fill Material	x Impervious Layer(s) X V	/eathered/Fractured Rock	X Bedrock
5.	Groundwater O	bserved: 🗌 Yes	x No	If yes:	N/A	N/A	
0.					Depth Wee	ping from Pit Depth St	anding Water in Hole
	Estimated Dept	h to High Groundwater:	108" inches	66.0 elevation	1		



Commonwealth of Massachusetts

City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (continued)

Deep Observation Hole Number:

TP-2

Depth (in.)	Soil Horizon/	Soil Matrix: Color-	Red	loximorphic Feat	ures	Soil Texture	Coarse Fragments % by Volume		Soil Structure	Soil	Other
Depth (m.)	Layer	Layer Moist (Munsell)	Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones		(Moist)	Other
0-8	А	10YR5/6				FSL					
8-24	В	10YR4/6				SL	5%	15%			
24-150	С	10YR3/6	108	7.5YR6/8	2%	S&G	5%	15%			

Additional Notes:

Fractured/weathered rock 60"-150". No water noted, no weeping.

ESHWT @ 108" (2% mottles - concentrations)

No Refusal



Commonwealth of Massachusetts City/Town of Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

	Sarkis Development Company						
	Owner Name 470 Boston Street					Map 2, Lot 5	5
	Street Address					Map/Lot #	
	Topsfield			MA		01983	
	City			State		Zip Code	
B	Site Information						
1.	(Check one) X New Constr	ruction	Upgrade		Repair		
2.	Soil Survey Available?	x Yes	🗌 No	If yes:	UC Davis Web Soil S	urvey	420B, 421C
	Canton Fine Sandy Loam			Bedroc	Source k		Soil Map Unit
	Soil Name			Soil Limita			
	Sandy till			Morra	line		
	Geologic/Parent Material			Landform			
3.	Surficial Geological Report Available?	🗌 Yes	X No	If yes:			
							Map Unit
4.	Flood Rate Insurance Map						
	Above the 500-year flood boundary? If Yes, continue to #5.	X Yes	🗌 No		e 100-year flood boundary A Zone A	/? X Yes	🗌 No
5.	Within a velocity zone?	Yes	X No				
6.	Within a Mapped Wetland Area?	X Yes	🗌 No	MassGI	S Wetland Data Layer: $^{ m W}$	Vooded Swamp Wetland Type	Deciduous/Mixe
7.	Current Water Resource Conditions	(USGS):	June, 2016 Month/Year	Range:	Above Normal	Normal X Bel	ow Normal
8.	Other references reviewed: N	J/A					



Commonwealth of Massachusetts City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (minimum of two holes required at every proposed primary and reserve disposal area)

	Deep Observat	ion Hole Number:	TP-3	7/7/2016 Date	8:00AM Time	Overcast, 65 degre Weather	ees
1.	Location		01.0			10 ((11/2) 50 020220	
	Ground Elevation	on at Surface of Hole:	81.0	Latitu	de/Longitude: ·	42.664163 / -70.930328	
	Description of L	ocation: Southwe	stern corner				
2.	Land Use	Open field			N/A		0-3%
		(e.g., woodland, agricultural fie Grass	ld, vacant lot, etc.)	Morraine	Surface Stones (e	e.g., cobbles, stones, boulders, etc.) $\mathrm{N/A}$	Slope (%)
		Vegetation		Landform	F	Position on Landscape (SU, SH, BS	, FS, TS)
3.	Distances from:	Open Water Body	N/A feet	Drainage Way	N/A feet	Wetlands	<u>500+ feet</u>
		Property Line	45'	_ Drinking Water V	Vell <u>N/A</u>	Other	N/A feet
4.	Parent Material:	Sandy till		Unsuital	ble Materials P	Present: X Yes	🗌 No
	If Yes:	Disturbed Soil	ill Material	x Impervious Layer(s)	X We	eathered/Fractured Rock	Bedrock
5.	Groundwater Ol	bserved: 🗌 Yes	x No	If yes:	N/A	N/A	
	Estimated Dept	h to High Groundwater:	102" nches	72.5 elevation	Depth Weep	ing from Pit Depth Stand	ling Water in Hole



Commonwealth of Massachusetts

City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (continued)

Deep Observation Hole Number:

TP-3

Depth (in.)	Soil Horizon/	il Horizon/ Soil Matrix: Color- Layer Moist (Munsell)	Red	loximorphic Feat	ures	Soil Texture		ragments /olume	Soil Structure	Soil	Other
Depth (m.)	Layer		Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones		(Moist)	Other
0-8	А	10YR4/4				FSL					
8-32	В	10YR3/6				LS		10%			
32-108	С	10YR3/6	102	7.5YR6/8	2%	LS		10%			Ref.@108"

Additional Notes:

No water noted, no weeping. ESHWT @ 102"

Angular cobbles and fractured rock throughout B & C layers

Refusal @ 108" (Ledge)



Commonwealth of Massachusetts City/Town of Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

	Owner Name 470 Boston Street			Map 2, Lot 5	
	Street Address			Map/Lot #	
	Topsfield		MA	01983	
	City		State	Zip Code	
В.	Site Information				
1.	(Check one) X New Construction	Upgrade	Repair		
2.	Soil Survey Available?	es 🗌 No	If yes: UC Davis Web Soil S	burvey	420B, 421C
			Source		Soil Map Unit
	Canton Fine Sandy Loam		Bedrock		
	Soil Name		Soil Limitations		
	Sandy till		Morraine		
	Geologic/Parent Material		Landform		
3.	Surficial Geological Report Available?	es X No	If yes:		
					Map Unit
4.	Flood Rate Insurance Map				
	Above the 500-year flood boundary? $\boxed{\mathbf{X}}$ Year flood boundary? $\boxed{\mathbf{X}}$ Year flood boundary?	es 🗌 No	Within the 100-year flood boundary FEMA Zone A	/? X Yes	🗌 No
5.	Within a velocity zone?	es 🛛 🗴 No			
6.	Within a Mapped Wetland Area? X	es 🗌 No	MassGIS Wetland Data Layer: $^{ m W}$	Vooded Swamp Wetland Type	Deciduous/Mix
7.	Current Water Resource Conditions (USG	S): June, 2016 Month/Year	Range: 🗌 Above Normal 🗌 I	Normal X Belo	ow Normal
8.	Other references reviewed: N/A				



Commonwealth of Massachusetts City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (minimum of two holes required at every proposed primary and reserve disposal area)

	Deep Observat	ion Hole Number:	TP-4	7/7/2016 Date	8:00AM Time	Overcast, 65 degree Weather	es
1.	Location						
	Ground Elevation	on at Surface of Hole:	73.0 feet	Latit	ude/Longitude	e: 42.664163 / -70.930328	_
	Description of L	ocation: Southw	est corner of p	property			
2.	Land Use	Open field			N/A		3-8%
		(e.g., woodland, agricultural fie Grass	eld, vacant lot, etc.)	Morraine	Surface Stones	(e.g., cobbles, stones, boulders, etc.) $N/A \label{eq:N}$	Slope (%)
		Vegetation		Landform		Position on Landscape (SU, SH, BS,	FS, TS)
3.	Distances from:	Open Water Body	N/A feet	_ Drainage Way	N/A feet	Wetlands	<u>500+ feet</u>
		Property Line	110' feet	_ Drinking Water	Well $\frac{N/A}{feet}$	Other	N/A feet
4.	Parent Material:	Sandy till		Unsuita	able Materials	Present: Yes	X No
	If Yes:	Disturbed Soil	Fill Material	Impervious Layer(s) 🗆 V	Neathered/Fractured Rock	Bedrock
5.	Groundwater O	oserved: <u>x</u> Yes	🗌 No	If yes:	N/A	<u>144"</u>	
	Estimated Dept	h to High Groundwater:	88" inches	65.7 elevation		eping from Pit Depth Standir	ng Water in Hole



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City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (continued)

Deep Observation Hole Number:

TP-4

Depth (in.)	Soil Horizon/	n/ Soil Matrix: Color- Moist (Munsell)	Redoximorphic Features			Soil Texture	Coarse Fragments % by Volume		Soil	Other
Depui (III.)	Layer		Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones	Consistence (Moist)	Other
0-10	А	10YR3/1				SL				
10-22	В	10YR3/2				SL				
22-80	B/C	10YR5/6				Sand				
80-144	С	10YR5/4	88	7.5YR5/8	5%	F. Sand				Moist

Additional Notes:

Standing water @ 144". ESHWT @ 88" (Some mottling in B/C transition layer - concentrations & depletions)

Concentrations - 5YR5/8, depletions - 10YR6/1

No Refusal.



Commonwealth of Massachusetts City/Town of Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

	Sarkis Development Company								
	Owner Name 470 Boston Street	Map 2, Lot 5							
	Street Address					Map/Lot #			
	Topsfield		MA		01983				
	City			State		Zip Code			
B	Site Information						<u> </u>		
1.	(Check one) X New Constru	uction	Upgrade		Repair				
2.	Soil Survey Available?	x Yes	🗌 No	If yes:	UC Davis Web Soil S Source	urvey	420B, 421C Soil Map Unit		
	Canton Fine Sandy Loam			Bedrock					
	Soil Name				Soil Limitations				
	Sandy till			Morra	ine				
	Geologic/Parent Material			Landform					
3.	Surficial Geological Report Available?	🗌 Yes	x No	If yes:					
							Map Unit		
4.	Flood Rate Insurance Map								
	Above the 500-year flood boundary? If Yes, continue to #5.	x Yes	🗌 No		e 100-year flood boundary A Zone A	? X Yes	🗌 No		
5.	Within a velocity zone?	🗌 Yes	X No						
6.	Within a Mapped Wetland Area?	X Yes	🗌 No	MassGI	S Wetland Data Layer: $^{ m W}$	Vooded Swamp Wetland Type	Deciduous/Mixe		
7.	Current Water Resource Conditions	(USGS):	June, 2016 Month/Year	Range:	Above Normal	Normal X Belo	ow Normal		
8.	Other references reviewed: N	/A							



Commonwealth of Massachusetts City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C	. On-Site Re	eview (minimum	of two holes red	quired at every pro	posed prin	nary and reserve dispose	al area)
	Deep Observa	tion Hole Number:	TP-5	7/7/2016	8:00AM	Overcast, 65 degr	rees
				Date	Time	Weather	
1.	Location						
	Ground Elevation	on at Surface of Hole	$\frac{70.0}{\text{feet}}$	Latitu	ude/Longitude	e: 42.664163 / -70.930328	
	Description of L	ocation: Cru		near end of paved di	riveway		
2.	Land Use	Open field			N/A		0-3%
		(e.g., woodland, agricult Grass	ural field, vacant lot, etc.) Morraine	Surface Stones	(e.g., cobbles, stones, boulders, etc N/A	:.) Slope (%)
		Vegetation		Landform		Position on Landscape (SU, SH, B	S, FS, TS)
3.	Distances from	: Open Water E	Body N/A	Drainage Way	N/A	Wetlands	375+ feet
			feet		feet		feet
		Property Line	220' feet	Drinking Water \	Well <u>N/A</u>	Other	N/A feet
4.	Parent Material	: Sandy till		Unsuita	ble Materials	Present: 🗌 Yes	X No
	If Yes:	Disturbed Soil	Fill Material	Impervious Layer(s)		Weathered/Fractured Rock	Bedrock
5.	Groundwater O	bserved: X Yes	s 🗌 No	If yes:	N/A	132"	
5.				ii yooi		eping from Pit Depth Star	nding Water in Hole
	Estimated Dept	h to High Groundwa	ter: 96"	62.0			-
		-	inches	elevation			



City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (continued)

Deep Observation Hole Number:

TP-5

Depth (in.)	Soil Horizon/	Soil Matrix: Color-	Rec	loximorphic Feat	ures	Soil Texture		ragments /olume	Soil Structure	Soil	Other
Depth (in.)	Layer	Moist (Munsell)	Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones		(Moist)	Other
0-8	А	10YR3/2				SL					
8-34	В	10YR3/1				SL					
34-72	2C1	10YR5/8				M. sand					Moist
72-132	2C2	10YR5/4	96	7.5YR6/8	5%	Sand			Loose, SG		

Additional Notes:

ESHWT @ 96" (5% mottles - concentrations & depletions)

Standing water @ 132" (bottom)

No Refusal, coarse sand layer at 60-70".



	Sarkis Development Company						
	Owner Name 470 Boston Street					Map 2, Lot 5	
	Street Address					Map/Lot #	
	Topsfield			MA		01983	
	City			State		Zip Code	
B	Site Information						
1.	(Check one) X New Construct	ion	Upgrade	[Repair		
2.	Soil Survey Available?	Yes	🗌 No	If yes:	UC Davis Web Soil S Source	urvey	420B, 421C Soil Map Unit
	Canton Fine Sandy Loam			Bedroc			Soli Map Onit
	Soil Name			Soil Limitat	tions		
	Sandy till			Morra	ine		
	Geologic/Parent Material			Landform			
3.	Surficial Geological Report Available?	Yes	X No	If yes:			Map Unit
4.	Flood Rate Insurance Map						
	Above the 500-year flood boundary? X If Yes, continue to #5.	Yes	🗌 No		e 100-year flood boundary A Zone A	? X Yes	🗌 No
5.	Within a velocity zone?	Yes	X No				
6.	Within a Mapped Wetland Area?	Yes	🗌 No	MassGIS	S Wetland Data Layer: $^{ m W}$	Vooded Swamp I Wetland Type	Deciduous/Mixe
7.	Current Water Resource Conditions (U		June, 2016 Month/Year	Range:	Above Normal	Normal X Belov	w Normal
8.	Other references reviewed: N/A						



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Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

С.	. On-Site Revie	W (minimum of t	wo holes req	uired at every pr	oposed pr	imary and reserve o	disposal a	area)
	Deep Observation	Hole Number:	TP-6	7/7/2016	8:00AM	Overcast,	65 degree	S
	•			Date	Time	Weather	C	
1.	Location							
	Ground Elevation at	t Surface of Hole:	70.5	Lati	tude/Longitu	de: 42. <u>664163 / -70.9</u>	930328	_
	Description of Locat	tion: Crushe		ear end of paved of	driveway			
2.		pen field			N/A			0-3%
	_	., woodland, agricultural fie rass	eld, vacant lot, etc.)	Morraine	Surface Stor	les (e.g., cobbles, stones, bol $\mathrm{N/A}$	ulders, etc.)	Slope (%)
	Veg	etation		Landform		Position on Landscape (SU, SH, BS, F	FS, TS)
3.	Distances from:	Open Water Body	N/A feet	_ Drainage Way	<u>N</u> feet			$\underbrace{175+feet}_{feet}$
		Property Line	<u>115'</u> feet	Drinking Water	Well <u>N</u>			N/A feet
4.	Parent Material:	Sandy till		Unsuit	able Materia	als Present:	Yes	X No
	If Yes: Distu	urbed Soil	Fill Material	Impervious Layer(s) 🗌	Weathered/Fractured Ro	ock	Bedrock
5.	Groundwater Obser	ved: 🗌 Yes	x No	If yes:	N/A		N/A	
				,		Veeping from Pit	Depth Standin	g Water in Hole
	Estimated Depth to	High Groundwater:	92"	62.8				-
	-		inches	elevatio	n			



City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (continued)

Deep Observation Hole Number:

TP-6

Depth (in.)	Soil Horizon/	Soil Matrix: Color-	Red	loximorphic Feat	ures	Soil Texture	Coarse F % by \	ragments /olume	Soil Structure	Soil	Other
Deptin (int.)	Layer	Moist (Munsell)	Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones		(Moist)	Canor
0-8	А	10YR4/3				FSL					
8-28	В	10YR6/6				FSL					
28-72	B/C	10YR3/3				FSL	5%	2%			
72-136	С	10YR3/3	92"	7.5YR6/8	2%	FSL	5%	2%			

Additional Notes:

ESHWT @ 92". No refusal, no weeping, no standing water.

Heavy manganese deposits throughout B/C and C layers, color 10R3/3



Owner Name 470 Boston Street			Map 2, Lot 5	
Street Address			Map/Lot #	
Topsfield		MA	01983	
City		State	Zip Code	
B. Site Information				
. (Check one) X New Construction	Upgrade	Repair		
2. Soil Survey Available?	es 🗌 No	If yes: UC Davis Web Soil Su	rvey	420B, 421C
		Source	<u> </u>	Soil Map Unit
Canton Fine Sandy Loam		Bedrock		
Soil Name		Soil Limitations		
Sandy till		Morraine		
Geologic/Parent Material		Landform		
8. Surficial Geological Report Available? 🗌 Ye	es <u>x</u> No	If yes:		
				Map Unit
 Flood Rate Insurance Map 				
Above the 500-year flood boundary? $\boxed{\mathbf{x}}$ Ye If Yes, continue to #5.	es 🗌 No	Within the 100-year flood boundary? FEMA Zone A	X Yes	🗌 No
5. Within a velocity zone?	es 🛛 🗴 No			
6. Within a Mapped Wetland Area? I Y	es 🗌 No	MassGIS Wetland Data Layer: $^{ m Wo}$	ooded Swamp Wetland Type	Deciduous/Mix
7. Current Water Resource Conditions (USG	S): June, 2016 Month/Year	Range: 🗌 Above Normal 🗌 No	ormal X Belo	ow Normal
3. Other references reviewed: N/A				



City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (minimum of two holes required at every proposed primary and reserve disposal area)

	Deep Observatio	n Hole Number:	TP-7	7/7/2016 Date	8:00AM Time	Overcast, 65 degree	es
1.	Location						
	Ground Elevation	at Surface of Hole:	72.0	Latitu	ide/Longitude	e: 42.664163 / -70.930328	_
	Description of Loc	cation: Woodla	nd area by we	etlands			
2.	Land Use	Woodland			N/A		0-3%
		.g., woodland, agricultural fie Grass	ld, vacant lot, etc.)) Morraine	Surface Stones	σ (e.g., cobbles, stones, boulders, etc.) $$N/A$$	Slope (%)
	V	egetation		Landform		Position on Landscape (SU, SH, BS,	FS, TS)
3.	Distances from:	Open Water Body	N/A feet	Drainage Way	<u>N/A</u> feet	Wetlands	<u>130+ feet</u>
		Property Line	250' feet	Drinking Water \	Vell <u>N/A</u>	Other	N/A feet
4.	Parent Material:	Sandy till		Unsuita	ble Materials	Present: 🗌 Yes	x No
	If Yes: 🗌 Di	sturbed Soil 🛛 🗌 F	Fill Material	Impervious Layer(s)		Weathered/Fractured Rock	Bedrock
5.	Groundwater Obs	erved: 🗌 Yes	x No	If yes:	N/A	N/A	
	Estimated Depth	o High Groundwater:	62" inches	66.8 elevation	Depth we	eping from Pit Depth Standi	ng Water in Hole



City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (continued)

Deep Observation Hole Number:

TP-7

Depth (in.)	Soil Horizon/	Soil Matrix: Color-	Rec	loximorphic Feat	ures	Soil Texture		ragments /olume		Soil Consistence	Other
Depth (In.)	Layer	Moist (Munsell)	Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones		(Moist)	Other
0-8	А	10YR3/3				FSL					
8-28	В	10YR5/6				FSL	5%	10%			
28-76	B/C	10YR4/4	62	7.5YR6/8	2%	FSL	5%	15%	Massive, fri.		
76-144	С	10YR4/4				FSL			Massive, fri.		5% cobbles

Additional Notes:

No water noted, no weeping. No refusal.

ESHWT @ 62" (Significant mottling - concentrations, color 7.5YR6/8)



	Owner Name 470 Boston Street			Map 2, Lot 5	
	Street Address			Map/Lot #	
	Topsfield		MA	01983	
	City		State	Zip Code	
В.	Site Information				
1.	(Check one) X New Construction	Upgrade	Repair		
2.	Soil Survey Available?	es 🗌 No	If yes: UC Davis Web Soil S	burvey	420B, 421C
			Source		Soil Map Unit
	Canton Fine Sandy Loam		Bedrock		
	Soil Name		Soil Limitations		
	Sandy till		Morraine		
	Geologic/Parent Material		Landform		
3.	Surficial Geological Report Available?	es X No	If yes:		
					Map Unit
4.	Flood Rate Insurance Map				
	Above the 500-year flood boundary? X Year If Yes, continue to #5.	es 🗌 No	Within the 100-year flood boundary FEMA Zone A	/? X Yes	🗌 No
5.	Within a velocity zone?	es 🛛 🗴 No			
6.	Within a Mapped Wetland Area? X	es 🗌 No	MassGIS Wetland Data Layer: $^{ m W}$	Vooded Swamp Wetland Type	Deciduous/Mix
7.	Current Water Resource Conditions (USG	S): June, 2016 Month/Year	Range: 🗌 Above Normal 🗌 I	Normal X Belo	ow Normal
8.	Other references reviewed: N/A				



Commonwealth of Massachusetts City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C.	On-Site Revie	W (minimum of tw	o holes requ	uired at eve	ry propose	ed primary ar	nd reserve d	disposal	area)
	Deep Observation H	Hole Number:	TP-8	7/7/2016	8:0	0AM	Overcast,	65 degree	es
	•			Date	Time		Weather		
1.	Location								
	Ground Elevation at	Surface of Hole:	68.5		Latitude/Lo	ongitude: 42. <u>66</u> 4	4163 / -70.9	30328	_
	Description of Location	on: Norther	n treeline, 50	' back corner	existing ga	arage			
2.	Land Use Op	en field			N/2	A			0-3%
		woodland, agricultural field	l, vacant lot, etc.)	Morraine	Surfa	ce Stones (e.g., cob N/A	bles, stones, bou	ulders, etc.)	Slope (%)
	Veget	tation		Landform		Position	on Landscape (S	SU, SH, BS, I	FS, TS)
3.	Distances from:	Open Water Body	N/A feet	Drainage	Way	N/A feet	Wetlands		$\underset{\text{feet}}{\underbrace{115+\text{ feet}}}$
		Property Line	<u>275'</u> feet	Drinking \	Water Well	N/A feet	Other		N/A feet
4.	Parent Material:	Sandy till		L	Insuitable M	laterials Presen	t: 🗌 Y	′es	x No
	If Yes: Distur	rbed Soil 🛛 🗌 Fi	Il Material] Impervious L	ayer(s)	U Weathere	ed/Fractured Ro	ock 🗌	Bedrock
5.	Groundwater Observ	ved: X Yes	🗌 No	If	yes:	N/A		150"	
••					, ee	Depth Weeping fron	n Pit D	Depth Standir	g Water in Hole
	Estimated Depth to H	High Groundwater:	108"		59.5			-	-
		in	ches	e	levation				



City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (continued)

Deep Observation Hole Number:

TP-8

Depth (in.)	Soil Horizon/	Soil Matrix: Color-	Red	oximorphic Feat	ures	Soil Texture	Coarse F % by \	ragments /olume	Soil Structure	Soil	Other
Depth (m.)	Layer	Moist (Munsell)	Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones		(Moist)	Other
0-8	А	10YR4/3				LS					
8-18	В	10YR56				LS	5%				
18-45	B/C	10YR5/8				LS	20%				
45-150	С	10YR5/6	108	7.5YR6/8	2%	Fine sand			Loose, SG		

Additional Notes:

ESHWT @ 108", some mottling, concentrations, color 7.5YR6/8

Standing water @ 150"

No Refusal. Well defined transition from LS to fine sand.



Owner Name 470 Boston Street			Map 2, Lot 5	
Street Address			Map/Lot #	
Topsfield		MA	01983	
City		State	Zip Code	
B. Site Information				
. (Check one) X New Construction	Upgrade	Repair		
2. Soil Survey Available?	es 🗌 No	If yes: UC Davis Web Soil Su	rvey	420B, 421C
		Source	<u> </u>	Soil Map Unit
Canton Fine Sandy Loam		Bedrock		
Soil Name		Soil Limitations		
Sandy till		Morraine		
Geologic/Parent Material		Landform		
8. Surficial Geological Report Available? 🗌 Ye	es <u>x</u> No	If yes:		
				Map Unit
 Flood Rate Insurance Map 				
Above the 500-year flood boundary? $\boxed{\mathbf{x}}$ Ye If Yes, continue to #5.	es 🗌 No	Within the 100-year flood boundary? FEMA Zone A	X Yes	🗌 No
5. Within a velocity zone?	es X No			
6. Within a Mapped Wetland Area? I Y	es 🗌 No	MassGIS Wetland Data Layer: $^{ m Wo}$	ooded Swamp Wetland Type	Deciduous/Mix
7. Current Water Resource Conditions (USG	S): June, 2016 Month/Year	Range: 🗌 Above Normal 🗌 No	ormal X Belo	ow Normal
3. Other references reviewed: N/A				



Commonwealth of Massachusetts City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C.	On-Site Review	N (minimum of two	vo holes req	uired at every pro	posed prim	ary and reserve dispose	al area)
	Deep Observation H	lole Number:	TP-9	7/7/2016	8:00AM	Overcast, 65 degr	rees
	·	_		Date	Time	Weather	
1.	Location						
	Ground Elevation at S	Surface of Hole:	70.0	Latitu	ude/Longitude:	42.664163 / -70.930328	
	Description of Location	on: Open fi	eld, proposed	detention basin			
2.	Land Use Op	en field			N/A		0-3%
	(e.g., v Gra	woodland, agricultural field	d, vacant lot, etc.)	Morraine	Surface Stones ((e.g., cobbles, stones, boulders, etc N/A	.) Slope (%)
	Vegeta	ation		Landform		Position on Landscape (SU, SH, B	S, FS, TS)
3.	Distances from:	Open Water Body	N/A feet	Drainage Way	<u>N/A</u> feet	Wetlands	<u>225+ feet</u>
		Property Line	<u>175'</u> feet	_ Drinking Water	Well <u>N/A</u>	Other	N/A feet
4.	Parent Material:	Sandy till		Unsuita	able Materials	Present: 🗌 Yes	x No
	If Yes: Disturb	bed Soil 🛛 🗌 Fi	II Material	Impervious Layer(s)) 🗆 W	/eathered/Fractured Rock [Bedrock
5.	Groundwater Observe	ed: 🗌 Yes	x No	If yes:	120"	138"	
0.				ii yes.			ding Water in Hole
	Estimated Depth to H	ligh Groundwater:	96"	62.0	·	'	-
		• _	nches	elevation	1		



City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (continued)

Deep Observation Hole Number:

TP-9

Depth (in.)	Soil Horizon/	Soil Matrix: Color-	Red	loximorphic Feat	ures	Soil Texture		ragments /olume	Soil Structure	Soil	Other
Depth (m.)	Layer	Moist (Munsell)	Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones		(Moist)	Other
0-8	А	10YR3/3				LS					
8-32	В	10YR5/4				LS					
32-72	2C1	10YR6/6				Sand					
72-138	2C2	10YR3/6	96	7.5YR6/8	2%	LS	10%				

Additional Notes:

Fractured/weathered rock throughout C layer. Standing water at 138", weeping at 120". ESHWT @ 96" (mottling)

No Refusal.



	Owner Name 470 Boston Street					Map 2, Lot 5	
	Street Address					Map/Lot #	
	_ Topsfield			MA		01983	
	City			State		Zip Code	
B	. Site Information						
1.	(Check one) X New Construct	ction	Upgrade] Repair		
2.	Soil Survey Available?	Yes	🗌 No	If yes:	UC Davis Web Soil Su	urvey	420B, 421C
	Canton Fine Sandy Loam			Bedrock	Source		Soil Map Unit
	Soil Name			Soil Limitatio			
	Sandy till			Morrai	ne		
	Geologic/Parent Material			Landform			
3.	Surficial Geological Report Available?	Yes	X No	If yes:			Map Unit
4.	Flood Rate Insurance Map						
	Above the 500-year flood boundary?	x Yes	🗌 No		100-year flood boundary Zone A	? X Yes	🗌 No
5.	Within a velocity zone?	Yes	X No				
6.	Within a Mapped Wetland Area?	x Yes	🗌 No	MassGIS	Wetland Data Layer: $^{ m W}$	ooded Swamp	Deciduous/Mixed
7.	Current Water Resource Conditions (L	JSGS):	Dec, 2016 Month/Year	Range:	Above Normal	lormal X Belo	w Normal
8.	Other references reviewed: N/2	А					



Commonwealth of Massachusetts City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C.	. On-Site Review (minimum of the	vo holes requ	uired at every pro	oposed prim	ary and reserve disposa	l area)
	Deep Observation Hole Number:	TP-10	12/9/2016	7:30AM	Overcast, 35 degr	ees
	•		Date	Time	Weather	
1.	Location					
	Ground Elevation at Surface of Hole:	68.0	Latit	ude/Longitude	42.664163 / -70.930328	
	Description of Location: Northea	st corner exist	ing garage			
2.	Land Use Open field			N/A		0-3%
	(e.g., woodland, agricultural fie Grass	d, vacant lot, etc.)	Morraine	Surface Stones	(e.g., cobbles, stones, boulders, etc. N/A) Slope (%)
	Vegetation		Landform		Position on Landscape (SU, SH, BS	S, FS, TS)
3.	Distances from: Open Water Body	N/A feet	Drainage Way	<u>N/A</u> feet	Wetlands	<u>120+/- feet</u>
	Property Line	150'+/- feet	Drinking Water	Well $\frac{N/A}{feet}$	Other	N/A feet
4.	Parent Material: Sandy till		Unsuit	able Materials	Present: 🗌 Yes	X No
	If Yes: Disturbed Soil F	ill Material] Impervious Layer(s) 🗆 V	/eathered/Fractured Rock	Bedrock
5.	Groundwater Observed: X Yes	🗌 No	If yes:	150"	154"	
0.					ping from Pit Depth Stan	ding Water in Hole
	Estimated Depth to High Groundwater:	150"	55.5		· - ·	-
		nches	elevation	า		



City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (continued)

Deep Observation Hole Number:

TP-10

Depth (in.)	Soil Horizon/	Soil Matrix: Color-	Red	loximorphic Feat	ures	Soil Texture	Coarse F % by \	ragments /olume	Soil Structure	Soil	Other
Depth (m.)	Layer	Moist (Munsell)	Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones		(Moist)	Other
0-18	А	10YR2/3				SL					
18-28	В	10YR5/6				LS	10%				
28-60	1C	10YR5/6				SL		2%			
60-156	2C	10YR5/6				SL		2%			

Additional Notes:

Water noted at 154", weeping at 150. ESHWT @ 150"

Cobbles and stones throughout B & C layers, no refusal,

layer fine sand at 60-70", no mottling noted.



	Owner Name 470 Boston Street					Map 2, Lot 5	
	Street Address					Map/Lot #	
	_ Topsfield			MA		01983	
	City			State		Zip Code	
B	. Site Information						
1.	(Check one) X New Construct	ction	Upgrade] Repair		
2.	Soil Survey Available?	Yes	🗌 No	If yes:	UC Davis Web Soil Su	urvey	420B, 421C
	Canton Fine Sandy Loam			Bedrock	Source		Soil Map Unit
	Soil Name			Soil Limitatio			
	Sandy till			Morrai	ne		
	Geologic/Parent Material			Landform			
3.	Surficial Geological Report Available?	Yes	X No	If yes:			Map Unit
4.	Flood Rate Insurance Map						
	Above the 500-year flood boundary?	x Yes	🗌 No		100-year flood boundary Zone A	? X Yes	🗌 No
5.	Within a velocity zone?	Yes	X No				
6.	Within a Mapped Wetland Area?	x Yes	🗌 No	MassGIS	Wetland Data Layer: $^{ m W}$	ooded Swamp	Deciduous/Mixed
7.	Current Water Resource Conditions (L	JSGS):	Dec, 2016 Month/Year	Range:	Above Normal	lormal X Belo	w Normal
8.	Other references reviewed: N/2	А					



Commonwealth of Massachusetts City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C.	. On-Site Re	eview (minimum of t	wo holes requ	uired at every pro	oposed prim	ary and reserve dispos	sal area)
	Deep Observa	tion Hole Number:	TP-11	12/9/2016	8:00AM	Overcast, 35 deg	grees
	•			Date	Time	Weather	
1.	Location						
	Ground Elevation	on at Surface of Hole:	69.0	Latit	ude/Longitude	42.664163 / -70.930328	<u>.</u>
	Description of L	_ocation:SouthW	est corner exis	sting garage			
2.	Land Use	Open field			N/A		0-3%
		(e.g., woodland, agricultural fie Grass	eld, vacant lot, etc.)	Morraine	Surface Stones	(e.g., cobbles, stones, boulders, e N/A	stc.) Slope (%)
		Vegetation		Landform		Position on Landscape (SU, SH,	BS, FS, TS)
3.	Distances from	: Open Water Body	N/A feet	_ Drainage Way	<u>N/A</u> feet	Wetlands	$\frac{120 + - feet}{feet}$
		Property Line	150'+/- feet	Drinking Water	Well <u>N/A</u>	Other	N/A feet
4.	Parent Material	: Sandy till		Unsuita	able Materials	Present: 🗌 Yes	X No
	If Yes:	Disturbed Soil	-ill Material] Impervious Layer(s)) 🗆 🛛	/eathered/Fractured Rock	Bedrock
5.	Groundwater O	bserved: x Yes	🗌 No	If yes:	156"	174"	
••						ping from Pit Depth Sta	anding Water in Hole
	Estimated Dept	th to High Groundwater:	156"	56.0			
			inches	elevation	1		



City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (continued)

Deep Observation Hole Number:

TP-11

Depth (in.)	Soil Horizon/	Soil Matrix: Color-	Red	oximorphic Feat	ures	Soil Texture		ragments /olume	Soil Structure	Soil	Other
Depth (m.)	Layer	Moist (Munsell)	Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones		(Moist)	Other
0-12	А	10YR2/3				SL					
12-36	B1	10YR5/4				LS					
36-66	B2	10YR5/6				Coarse sand	10-15%		Loose, SG		
66-120	1C	10YR5/8				Sand			Loose, SG		
120-180	2C	10YR5/8				LS					

Additional Notes:

Standing Water noted at 174", weeping at 156. ESHWT

@ 156", no refusal, no mottling noted.



	Owner Name 470 Boston Street					Map 2, Lot 5	
	Street Address					Map/Lot #	
	_ Topsfield			MA		01983	
	City			State		Zip Code	
B	. Site Information						
1.	(Check one) X New Construct	ction	Upgrade] Repair		
2.	Soil Survey Available?	Yes	🗌 No	If yes:	UC Davis Web Soil Su	urvey	420B, 421C
	Canton Fine Sandy Loam			Bedrock	Source		Soil Map Unit
	Soil Name			Soil Limitatio			
	Sandy till			Morrai	ne		
	Geologic/Parent Material			Landform			
3.	Surficial Geological Report Available?	Yes	X No	If yes:			Map Unit
4.	Flood Rate Insurance Map						
	Above the 500-year flood boundary?	x Yes	🗌 No		100-year flood boundary Zone A	? X Yes	🗌 No
5.	Within a velocity zone?	Yes	X No				
6.	Within a Mapped Wetland Area?	x Yes	🗌 No	MassGIS	Wetland Data Layer: $^{ m W}$	ooded Swamp	Deciduous/Mixed
7.	Current Water Resource Conditions (L	JSGS):	Dec, 2016 Month/Year	Range:	Above Normal	lormal X Belo	w Normal
8.	Other references reviewed: N/2	А					



Commonwealth of Massachusetts City/Town of

Estimated Depth to High Groundwater:

144''

inches

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

С	. On-Site Review (minimum of	two holes re	equired at every	proposed primary	/ and reserve disposal area)
	Deep Observation Hole Number:	TP-12	12/9/2016	8:30AM	Overcast, 35 degrees
			Date	Time	Weather
1.	Location				
		(0.0	_		C(41(2), 70, 020220)

	Ground Elevation	on at Surface of	Hole:	68.0	Latitu	ide/Longitude	+42.664163 / -/0.9	30328	
	Description of L	ocation:	North w	vest corner pr	oposed UIS-2, alor	ng treeline			
2.	Land Use	Open field				N/A			0-3%
		(e.g., woodland, as Grass	gricultural field	, vacant lot, etc.)	Morraine	Surface Stones	(e.g., cobbles, stones, bou N/A	Ilders, etc.)	Slope (%)
		Vegetation			Landform		Position on Landscape (S	U, SH, BS, F	S, TS)
3.	Distances from	Open Wa	ater Body	N/A feet	Drainage Way	$\frac{N/A}{feet}$	Wetlands		$\frac{100 + - feet}{feet}$
		Property	Line	300'+/-	Drinking Water \	Well <u>N/A</u>	Other		N/A feet
4.	Parent Material	: Sandy t	ill		Unsuita	ble Materials	Present: Y	′es	X No
	If Yes:	Disturbed Soil	🗌 Fil	I Material] Impervious Layer(s)	□ v	Veathered/Fractured Rc	ock	Bedrock
5.	Groundwater O	bserved: X	Yes	🗌 No	If yes:	144"		158"	
					•	Depth Wee	eping from Pit D	epth Standing	g Water in Hole

56.0

elevation



City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (continued)

Deep Observation Hole Number:

TP-12

Depth (in.)	Soil Horizon/	Soil Matrix: Color- Moist (Munsell)	Redoximorphic Features			Soil Texture	Coarse F % by \	Fragments Volume		Soil Consistence	Other	
Depth (m.)	Layer		Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones		(Moist)	Other	
0-6	A	10YR2/3				SL						
6-26	Bw	10YR5/4				LS	5-7%					
26786	1C	10YR5/6				Med. sand			Loose, SG		Boulder boundar	-
78-162	2C	10YR5/8				Sand					Angular	cobbles

Additional Notes:

Standing Water noted at 158", weeping at 144.

ESHWT @ 144", no refusal, very little/no mottling

noted. Some fractured ledge at 160", easily broken up.



	Owner Name 470 Boston Street					Map 2, Lot 5	
	Street Address					Map/Lot #	
	_ Topsfield			MA		01983	
	City			State		Zip Code	
B	. Site Information						
1.	(Check one) X New Construct	ction	Upgrade] Repair		
2.	Soil Survey Available?	Yes	🗌 No	If yes:	UC Davis Web Soil Su	urvey	420B, 421C
	Canton Fine Sandy Loam			Bedrock	Source		Soil Map Unit
	Soil Name			Soil Limitatio			
	Sandy till			Morrai	ne		
	Geologic/Parent Material			Landform			
3.	Surficial Geological Report Available?	Yes	X No	If yes:			Map Unit
4.	Flood Rate Insurance Map						
	Above the 500-year flood boundary?	x Yes	🗌 No		100-year flood boundary Zone A	? X Yes	🗌 No
5.	Within a velocity zone?	Yes	X No				
6.	Within a Mapped Wetland Area?	x Yes	🗌 No	MassGIS	Wetland Data Layer: $^{ m W}$	ooded Swamp	Deciduous/Mixed
7.	Current Water Resource Conditions (L	JSGS):	Dec, 2016 Month/Year	Range:	Above Normal	lormal X Belo	w Normal
8.	Other references reviewed: N/2	А					



City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (minimum of two holes required at every proposed primary and reserve disposal area)

	Deep Observat	ion Hole Number:	TP-13	12/9/2016 Date	9:30AM Time	Overcast, 35 degree Weather	25
1.	Location						
	Ground Elevation	on at Surface of Hole:	72.5	Latit	ude/Longitude	e: 42.664163 / -70.930328	_
	Description of L	ocation: South	east corner (Se	e Test Pits Plan, T	PP-1)		
2.	Land Use	Open field			N/A		3-8%
		(e.g., woodland, agricultural fin	eld, vacant lot, etc.)	Morraine	Surface Stones	$(\mbox{e.g.},\mbox{ cobbles},\mbox{ stones},\mbox{ boulders},\mbox{ etc.})} N/A$	Slope (%)
		Vegetation		Landform		Position on Landscape (SU, SH, BS, I	FS, TS)
3.	Distances from:	Open Water Body	N/A feet	_ Drainage Way	<u>N/A</u> feet	Wetlands	$\frac{450 + - feet}{feet}$
		Property Line	100'+/	_ Drinking Water	Well <u>N/A</u>	Other	N/A feet
4.	Parent Material:	Sandy till		Unsuit	able Materials	Present: Yes	X No
	If Yes:	Disturbed Soil	Fill Material [Impervious Layer(s) 🗆 \	Neathered/Fractured Rock	Bedrock
5.	Groundwater Ol	bserved: X Yes	🗌 No	If yes:	100"	122"	
			_	,	Depth We	eping from Pit Depth Standir	ng Water in Hole
	Estimated Dept	h to High Groundwater:	122"	62.3			
			inches	elevation	ו		



City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (continued)

Deep Observation Hole Number:

TP-13

Depth (in.)	Soil Horizon/	Soil Matrix: Color- Moist (Munsell)	Redoximorphic Features			Soil Texture	Coarse F % by \	ragments /olume		Soil	Other	
Depth (m.)	Layer		Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones	Soil Structure	(Moist)	Other	
0-10	А	10YR2/3				FSL						
10-40	Bw	10YR5/4				FSL						
40-132	С	10YR5/6				FSL		2%	Massive, fri	able	Some frac	tured
											ledge	

Additional Notes:

Standing Water noted at 122", weeping at 100. ESHWT @ 100", no

refusal, very little/no mottling noted. Some fractured ledge in C

horizon, easily broken up.



	Owner Name 470 Boston Street					Map 2, Lot 5	
	Street Address					Map/Lot #	
	_ Topsfield			MA		01983	
	City			State		Zip Code	
B	. Site Information						
1.	(Check one) X New Construct	ction	Upgrade] Repair		
2.	Soil Survey Available?	Yes	🗌 No	If yes:	UC Davis Web Soil Su	urvey	420B, 421C
	Canton Fine Sandy Loam			Bedrock	Source		Soil Map Unit
	Soil Name			Soil Limitatio			
	Sandy till			Morrai	ne		
	Geologic/Parent Material			Landform			
3.	Surficial Geological Report Available?	Yes	X No	If yes:			Map Unit
4.	Flood Rate Insurance Map						
	Above the 500-year flood boundary?	x Yes	🗌 No		100-year flood boundary Zone A	? X Yes	🗌 No
5.	Within a velocity zone?	Yes	X No				
6.	Within a Mapped Wetland Area?	x Yes	🗌 No	MassGIS	Wetland Data Layer: $^{ m W}$	ooded Swamp	Deciduous/Mixed
7.	Current Water Resource Conditions (L	JSGS):	Dec, 2016 Month/Year	Range:	Above Normal	lormal X Belo	w Normal
8.	Other references reviewed: N/2	А					



City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (minimum of two holes required at every proposed primary and reserve disposal area)

	Deep Observa	tion Hole Number:	TP-14	12/9/2016 Date	10:30AM Time	Overcast, 35 degree Weather	28
1.	Location						
	Ground Elevation	on at Surface of Hole:	72.5	Latit	ude/Longitude	e: 42.664163 / -70.930328	_
	Description of L	ocation: South	ern corner (Se	e Test Pits Plan, TI	PP-1)		
2.	Land Use	Open field			N/A		3-8%
		(e.g., woodland, agricultural fi $$Grass$$	eld, vacant lot, etc.)	Morraine	Surface Stones	(e.g., cobbles, stones, boulders, etc.)	Slope (%)
~		Vegetation	N/A	Landform		Position on Landscape (SU, SH, BS, F	
3.	Distances from:	Open Water Body	feet	_ Drainage Way	<u>N/A</u> feet	Wetlands	$\frac{430+-\text{feet}}{\text{feet}}$
		Property Line	125'+/	- Drinking Water	Well <u>N/A</u>	Other	N/A feet
4.	Parent Material	Sandy till		Unsuit	able Materials	Present: Yes	X No
	If Yes:	Disturbed Soil	Fill Material	Impervious Layer(s) 🗆 '	Neathered/Fractured Rock	Bedrock
5.	Groundwater O	bserved: <u>x</u> Yes	🗌 No	If yes:	118"	122"	
	Estimated Dept	h to High Groundwater:	122" inches	62.7 elevation		eping from Pit Depth Standin	g Water in Hole



City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (continued)

Deep Observation Hole Number:

TP-14

Depth (in.)	Soil Horizon/	Soil Matrix: Color-	Red	oximorphic Featu	ures	Soil Texture	Coarse F % by \	ragments /olume	Soil Structure	Soil	Other
Depth (m.)	Layer	Moist (Munsell)	Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones		(Moist)	Other
0-12	А	10YR2/3				LS					
12-36	Bw	10YR5/4				LS					
36-132	С	10YR5/6				FSL		2%	Massive, fri	able	

Additional Notes:

Standing Water noted at 122", weeping at 118. ESHWT @ 118",

no refusal, very little/no mottling noted.



	Owner Name 470 Boston Street			Map 2, Lot 5					
	Street Address					Map/Lot #			
	_ Topsfield			MA		01983			
	City			State		Zip Code			
B	. Site Information								
1.	(Check one) X New Constru-	ction	Upgrade	Γ	Repair				
2.	Soil Survey Available?	Yes	🗌 No	If yes:	UC Davis Web Soil S	urvey	420B, 421C		
	Canton Fine Sandy Loam			Source Soil Map Unit Bedrock					
	Soil Name			Soil Limitati					
	Sandy till			Morrai	ne				
	Geologic/Parent Material			Landform					
3.	Surficial Geological Report Available?	Yes	X No	If yes:			Map Unit		
4.	Flood Rate Insurance Map								
	Above the 500-year flood boundary? [If Yes, continue to #5.	x Yes	🗌 No		e 100-year flood boundary A Zone A	? X Yes	🗌 No		
5.	Within a velocity zone?	Yes	X No						
6.	Within a Mapped Wetland Area?	x Yes	🗌 No	MassGIS	Wetland Data Layer: $^{\mathrm{W}}$	ooded Swamp	Deciduous/Mixed		
7.	Current Water Resource Conditions (USGS):	Dec, 2016 Month/Year	Range:	Above Normal	lormal 🛛 Belo	w Normal		
8.	Other references reviewed: N/	А							



City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (minimum of two holes required at every proposed primary and reserve disposal at	rea)
--	------

	Deep Observa	tion Hole Number:	TP-15	12/9/2016 Date	11:00AM Time	Overcast, 35 degr Weather	rees
1.	Location						
	Ground Elevation	on at Surface of Hole:	72.2	Lat	itude/Longitud	e: 42.664163 / -70.930328	
	Description of L	ocation: South	ern corner (Se	e Test Pits Plan, T	'PP-1)		
2.	Land Use	Open field			N/A		3-8%
		(e.g., woodland, agricultural fie Grass	eld, vacant lot, etc.)	Morraine	Surface Stones	s (e.g., cobbles, stones, boulders, etc $\mathrm{N/A}$.) Slope (%)
		Vegetation		Landform		Position on Landscape (SU, SH, B	S, FS, TS)
3.	Distances from	Open Water Body	N/A feet	_ Drainage Way	r <u>N/A</u> feet	A Wetlands	<u>300+/- feet</u>
		Property Line	125'+/	- Drinking Wate	r Well <u>N/A</u>	A Other	N/A feet
4.	Parent Material	: Sandy till		Unsui	itable Materials	s Present: 🗌 Yes	X No
	If Yes:	Disturbed Soil	Fill Material	Impervious Layer	(s)	Weathered/Fractured Rock	Bedrock
5.	Groundwater O	bserved: X Yes	🗌 No	If yes:	: 120"	136"	
						eeping from Pit Depth Star	ding Water in Hole
	Estimated Dept	h to High Groundwater:	120	62.2	•		-
	·	-	inches	elevatio	on	-	



City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (continued)

Deep Observation Hole Number:

TP-15

Depth (in.)	Soil Horizon	Soil Matrix: Color-	Rec	loximorphic Feat	ures	Soil Texture		ragments /olume		Soil Consistence	Other			
Depth (m.)	Layer	Moist (Munsell)	Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones				(Moist)	Other	
0-12	А	10YR2/3				FSL								
12-40	Bw	10YR5/4				FSL								
40-140	С	10YR5/6				SL		5%	Massive, fri	able	Angular	cobbles		

Additional Notes:

Standing Water noted at 136", weeping at 120. ESHWT @ 120", partial refusal west

side of pit @116", mottling noted below weep line. Some fine materials in C layer.

Some angular cobbles in C layer (~5%)



	Owner Name 470 Boston Street			Map 2, Lot 5					
	Street Address					Map/Lot #			
	_ Topsfield			MA		01983			
	City			State		Zip Code			
B	. Site Information								
1.	(Check one) X New Constru-	ction	Upgrade	Γ	Repair				
2.	Soil Survey Available?	Yes	🗌 No	If yes:	UC Davis Web Soil S	urvey	420B, 421C		
	Canton Fine Sandy Loam			Source Soil Map Unit Bedrock					
	Soil Name			Soil Limitati					
	Sandy till			Morrai	ne				
	Geologic/Parent Material			Landform					
3.	Surficial Geological Report Available?	Yes	X No	If yes:			Map Unit		
4.	Flood Rate Insurance Map								
	Above the 500-year flood boundary? [If Yes, continue to #5.	x Yes	🗌 No		e 100-year flood boundary A Zone A	? X Yes	🗌 No		
5.	Within a velocity zone?	Yes	X No						
6.	Within a Mapped Wetland Area?	x Yes	🗌 No	MassGIS	Wetland Data Layer: $^{\mathrm{W}}$	ooded Swamp	Deciduous/Mixed		
7.	Current Water Resource Conditions (USGS):	Dec, 2016 Month/Year	Range:	Above Normal	lormal 🛛 Belo	w Normal		
8.	Other references reviewed: N/	А							



City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (minimum of two holes required at every proposed primary and reserve disposal area)

	Deep Observat	ion Hole Number:	TP-16	12/9/2016 Date	11:30AM Time	Overcast, 35 degree Weather	S
1.	Location						
	Ground Elevation	on at Surface of Hole:	68.0	Latit	tude/Longitude	e: 42.664163 / -70.930328	_
	Description of L	ocation: Center	r of site (See T	est Pits Plan, TPP-	-1)		
2.	Land Use	Open field			N/A		0-3%
		(e.g., woodland, agricultural fie Grass	eld, vacant lot, etc.)	Morraine	Surface Stones	(e.g., cobbles, stones, boulders, etc.) N/A	Slope (%)
_		Vegetation	NT / A	Landform		Position on Landscape (SU, SH, BS, F	
3.	Distances from:	Open Water Body	N/A feet	_ Drainage Way	<u>N/A</u> feet	Wetlands	$\frac{200+-\text{feet}}{\text{feet}}$
		Property Line	180'+/	- Drinking Water	Well $\frac{N/A}{feet}$	Other	N/A feet
4.	Parent Material:	Sandy till		Unsuit	able Materials	Present: Yes	X No
	If Yes:	Disturbed Soil	Fill Material	Impervious Layer(s	s) 🗌 \	Neathered/Fractured Rock	Bedrock
5.	Groundwater Ol	oserved: <u>x</u> Yes	🗌 No	If yes:	120"	128"	
	Estimated Dast		120	58.0	Depth We	eping from Pit Depth Standin	g Water in Hole
		n to High Groundwater:	120 inches				



City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (continued)

Deep Observation Hole Number:

TP-16	
-------	--

Depth (in.)	Soil Horizon	Soil Matrix: Color-	Rec	loximorphic Feat	ures	Soil Texture		ragments /olume	Soil Structure	Soil Consistence	Other		
Depth (m.)	Layer	Moist (Munsell)	Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones	5		(Moist)	Other	
0-18	A	10YR2/3				FSL							
18-24	Bw	10YR5/4				FSL						_	
24-60	1C	10YR5/6				Sand						_	
60-138	2C	10YR5/6				SL		5%			Angular	cobbles	
												_	

Additional Notes:

Standing Water noted at 128", weeping at 120. ESHWT @ 120", refusal @

138" (bedrock), Some angular cobbles in C layer (~5%)



	Owner Name 470 Boston Street			Map 2, Lot 5					
	Street Address					Map/Lot #			
	_ Topsfield			MA		01983			
	City			State		Zip Code			
B	. Site Information								
1.	(Check one) X New Constru-	ction	Upgrade	Γ	Repair				
2.	Soil Survey Available?	Yes	🗌 No	If yes:	UC Davis Web Soil S	urvey	420B, 421C		
	Canton Fine Sandy Loam			Source Soil Map Unit Bedrock					
	Soil Name			Soil Limitati					
	Sandy till			Morrai	ne				
	Geologic/Parent Material			Landform					
3.	Surficial Geological Report Available?	Yes	X No	If yes:			Map Unit		
4.	Flood Rate Insurance Map								
	Above the 500-year flood boundary? [If Yes, continue to #5.	x Yes	🗌 No		e 100-year flood boundary A Zone A	? X Yes	🗌 No		
5.	Within a velocity zone?	Yes	X No						
6.	Within a Mapped Wetland Area?	x Yes	🗌 No	MassGIS	Wetland Data Layer: $^{\mathrm{W}}$	ooded Swamp	Deciduous/Mixed		
7.	Current Water Resource Conditions (USGS):	Dec, 2016 Month/Year	Range:	Above Normal	lormal 🛛 Belo	w Normal		
8.	Other references reviewed: N/	А							



Commonwealth of Massachusetts City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (minimum of two holes required at every proposed primary and reserve disposal area) Deep Observation Hole Number: <u>TP-17</u> <u>12/9/2016</u> <u>12:30AM</u> <u>Overcast, 35 degrees</u> <u>Time</u> <u>Weather</u>

	-	D		ime	weather	
1.	Location					
	Ground Elevation at Surface of Hole:	69.5	Latitude	e/Longitude: 42	.664163 / -70.930328	3
	Description of Location: Souther	n corner of exi	sting garage (See T	est Pits Plan, T	TPP-1)	
2.	Land Use Open field		1	N/A		0-3%
	(e.g., woodland, agricultural field Grass		Morraine		cobbles, stones, boulders, $\langle A$	etc.) Slope (%)
	Vegetation	La	andform	Pos	ition on Landscape (SU, SH	BS, FS, TS)
3.	Distances from: Open Water Body	N/A feet	Drainage Way	<u>N/A</u> feet	Wetlands	<u>150+/- feet</u>
	Property Line	$\frac{160'+/-}{\text{feet}}$	Drinking Water We		Other	<u>N/A</u> feet
4.	Parent Material: Sandy till		Unsuitabl	e Materials Pre	sent: 🗌 Yes	X No
	If Yes: Disturbed Soil Fil	l Material	Impervious Layer(s)	🗌 Weatl	nered/Fractured Rock	Bedrock
5.	Groundwater Observed: X Yes	🗌 No	If yes:	156" Depth Weeping	from Pit Dopth S	tanding Water in Hole
	· · · ·	156 ches	56.5 elevation			



Commonwealth of Massachusetts

City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (continued)

Deep Observation Hole Number:

TP-17

Depth (in.)	Soil Horizon	Soil Horizon/	Soil Matrix: Color-	Soil Matrix: Color-	Soil Matrix: Color	Redoximorphic Features		Soil Texture	Coarse Fragments % by Volume		s Soil Structure	Soil	e Other	
Depth (m.)	Layer	Moist (Munsell)	Depth	Color		Cobbles & Stones		(Moist)	Other					
0-20	A	10YR2/1				LS					Heavily p	lowed		
20-36	Bw	10YR5/8				SL								
36-60	1C	10YR5/6				Med. Sand		5%			Some coa	rse sand		
60-168	2C	10YR5/6				SL					Angular	cobbles		
											manganes	e deposits		

Additional Notes:

Standing Water noted at 166", weeping at 156. ESHWT @ 156", refusal

@ 168" (bedrock), Some angular cobbles in 1C layer (~5%)

15 Re	esear	ngland E ch Drive MA 01002	nvironmer	A DIVISION OF	PROJECT: LOCATION: Project No.:	470 Boston St Topsfield, MA 039312,00	TESTPIT NO.: SHEET NO.: 1 DATE:	SWTP-1 of 1 9-Dec-16
(413)			NEE	SWCA	Subcontractor:	T.W. Excavating hired by Allen &	NEE Engineer/Geolo	
						Major Associates, Inc.	Ū	-
			WATER READIN		G.S. ELEVATION:	EL 68.25 (estimated)		
DATE	204.0	TIME	DEPTH to	GW	DATUM: LOCATION:	between TP-16-4 and TP-16-3		
Dec. 9	i ľ				LOCATION.	between TF-10-4 and TF-10-3		
DEPTH	Р Н О Т О	Ir			SAMPLE DESC	RIPTION		STRATUM DESCRIPTION
Ĥ	Ó #	ID	(ft) PID	Analysis				
					0 - 0.5' Top Soil, S	andy Loam		LOAM
					1			
					1			
					1			
2	\vdash			1	1			
				1	0.5 - 5' brown Find	e to Coarse SAND, little gravel, trace silt		OUTWASH
	┝─┤				0.5 - 5 brown, rine	to coarse same, inte graver, trace sin		
	\square				4			DEPOSIT
4	\vdash				4			
	\square				4			
	\square				 			
					1			
6					1			
÷					1			
8					Ţ			
0					1			
					1			
					5 - 14.5' brown CO	ARSE SAND AND GRAVEL, trace silt,	moist to wet, crudely	SAND WITH GRAVEL
					stratified with angu	lar clasts	-	(Ablation Till)
10					1			(
					1			
					1			
	\vdash			1	1			
12	┝─┤			1	1			
	┝─┤				1			
					1			
	┝─┤				1			
14	┝─┤				1			
	┝─┤				1			
					4			END OF EXPLORATION
	\square				4			Refusal @ 14.5'
16	⊢┤				┨─────			
					<u> </u>			
CROS	S-SEC	TION:			REMARKS/NOTES	S:		
					#1 area			
								SWTP-1

15 Re	esear erst, 1	ch Drive MA 01002 0202	nvironmer NEE	A DIVISION OF SWCAA	PROJECT: LOCATION: Project No.: Subcontractor:	470 Boston St Topsfield, MA 039312,00 T.W. Excavating hired by Allen & Major Associates, Inc.	TESTPIT NO.: SHEET NO.: 1 DATE: NEE Engineer/Geolo	SWTP-2 of 1 9-Dec-16 gist: JPJ
DATE			DWATER READIN		G.S. ELEVATION: DATUM:	EL 71.0 (estimated)		
DATE Dec. 9	2016	11:30 AN			LOCATION:	between TP-16-5 & 6 and TP-16-7 8	8	
DCC. 3 D E P T H	P H O T O	SAM			, SAMPLE DESC			STRATUM DESCRIPTION
	#		(ft)					10111
2					0 - 0.5' Top Soil, S	anoy Loam		LOAM
					0.5 - 4' brown, Fine	to Coarse SAND, little gravel, trace si	lt	OUTWASH DEPOSIT
4								
8					4 - 12 0' brown CO	ARSE SAND AND GRAVEL, trace to	ittle silt mojet to	SAND WITH GRAVEL
10						ratified with angular clasts	nue sit, moist te	(Ablation Till)
12								END OF EXPLORATION Refusal @ 12.0'
14								
16 _{CROS}					REMARKS/NOTES	<u>.</u>		
	0-0EU				In SSDS #3 area			SWTP-2

15 R	esear erst,	ngland H ch Drive MA 01002 0202	1	onmen	A DIVISION OF	PROJECT: LOCATION: Project No.: Subcontractor:	470 Boston St Topsfield, MA 039312,00 T.W. Excavating hired by Allen &	TESTPIT NO.: SHEET NO.: 1 DATE: NEE Engineer/Geolog	SWTP-3 of 1 9-Dec-16 gist: JPJ
			V	Ì			Major Associates, Inc.		
				R READING		G.S. ELEVATION:	EL 65.0 (estimated)		
DATE		TIME		DEPTH to		DATUM:			
Dec. 9	, 2016	12:00 PI	М	None Obse	erved	LOCATION:	near TP-16-9		
D E P T H	PHOTO			LLECTION	DATA	SAMPLE DESC	RIPTION		STRATUM DESCRIPTION
н	o v	ID	DEPTH (ft)	PID	Analysis				DESCRIPTION
	#		(14)				and at a sec		LOAM
						0 - 0.5' Top Soil, S	andy Loam		LUAM
						1			
2						4			
	┝──┤					4			
	\vdash					0.5 - 4' brown, Fine	to Coarse SAND, little gravel, trace silt		OUTWASH
1						1			DEPOSIT
4						1		Ī	
	┝─┤					1 6 0' brown 00 6		oilt maint to	
	┝──┤						RSE SAND AND GRAVEL, trace to little ratified with angular clasts	e siit, moist to	SAND WITH GRAVEL
	\square					. moist, crudely St	นแกรน พนา สามุนเลา เเสอเอ		(Ablation Till)
6									
0						Ĩ			END OF EXPLORATION
						1			Refusal @ 6.0'
						4			
						4			
8						4			
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10						4			
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						1			
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CROS	S-SEC	CTION:				REMARKS/NOTES			
						In SSDS #2 area			
									SWTP-3
1									
						1			

ATTACHMENT B

Grain Size Analyses

H





Prepared For:

Jack Jemsek SWCA Incorporated 15 Research Drive Amherst, MA 01002

jjemsek@swca.com 413-658-2055

Soil and Plant Nutrient Testing Laboratory

203 Paige Laboratory 161 Holdsworth Way University of Massachusetts Amherst, MA 01003 Phone: (413) 545-2311 e-mail: soiltest@umass.edu website: soiltest.umass.edu

Sample Information:

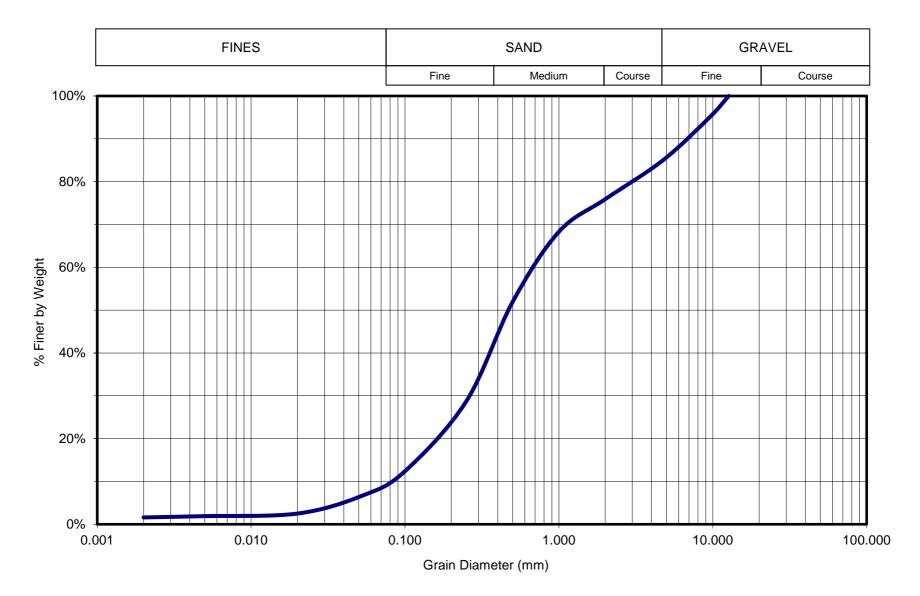
Sample ID: SWTP 1, 3 to 5 '

26938
X161219-101
12/19/2016
12/22/2016

USDA Size Fraction			Per	rcent of W	hole Sample Passing
Main Fractions	<u>Size (mm)</u> 0.05-2.0	Percent 91.2	<u>Size (mm)</u> 2.00	<u>Sieve #</u> #10	Whole Sample % of Sample Passing 75.9
Silt Clay	0.002-0.05 <0.002	6.7 2.1	1.00 0.50 0.25	#18 #35 #60	68.3 51.9 28.7
<u>Sand Fractions</u> Very Coarse	<u>Size (mm)</u> 1.0-2.0	Percent 10.1	0.10 0.053	#140 #270	12.4 6.7
Coarse Medium Fine	0.5-1.0 0.25-0.5 0.10-0.25	21.5 30.6 21.5	0.02 0.005 0.002	20 um 5 um 2 um	2.5 1.9 1.6
Very Fine	0.05-0.10	7.5			
<u>Silt Fractions</u> Coarse Medium	<u>Size (mm)</u> 0.02-0.05 0.005-0.02	<u>Percent</u> 5.6 0.7			
Fine	0.002-0.005	0.4			

USDA Textural Class: coarse sand

Gravel Content: (%) 24.1



Grain size analysis for SWTP1, 3 to 5'



Prepared For:

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Sample Information:

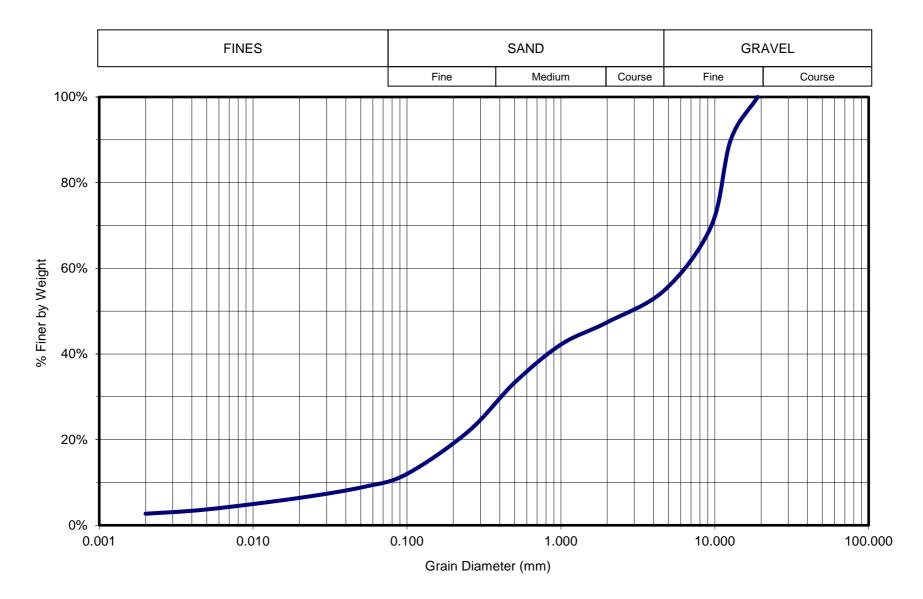
Sample ID: SWTP 1 8 to 9 '

Order Number:	26938
Lab Number:	X161219-102
Received:	12/19/2016
Reported:	12/22/2016

USDA Size Fraction			Pe	rcent of W	hole Sample Passing
<u>Main Fractions</u> Sand Silt Clay	<u>Size (mm)</u> 0.05-2.0 0.002-0.05 <0.002	Percent 81.1 13.3 5.6	<u>Size (mm)</u> 2.00 1.00 0.50	<u>Sieve #</u> #10 #18 #35	Whole Sample % of Sample Passing 47.4 42.2 33.3
Sand Fractions Very Coarse Coarse Medium Fine Very Fine	Size (mm) 1.0-2.0 0.5-1.0 0.25-0.5 0.10-0.25 0.05-0.10	Percent 10.9 18.8 24.0 20.8 6.5	0.25 0.10 0.053 0.02 0.005 0.005	#60 #140 #270 20 um 5 um 2 um	21.9 12.0 9.0 6.4 3.7 2.7
<u>Silt Fractions</u> Coarse Medium Fine	Size (mm) 0.02-0.05 0.005-0.02 0.002-0.005	Percent 5.4 5.7 2.1			

USDA Textural Class: gravelly loamy coarse sand

Gravel Content: (%) 52.6



Grain size analysis for SWTP1, 8 to 9'



Prepared For:

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Sample Information:

Sample ID: SWTP 1 11 to 12'

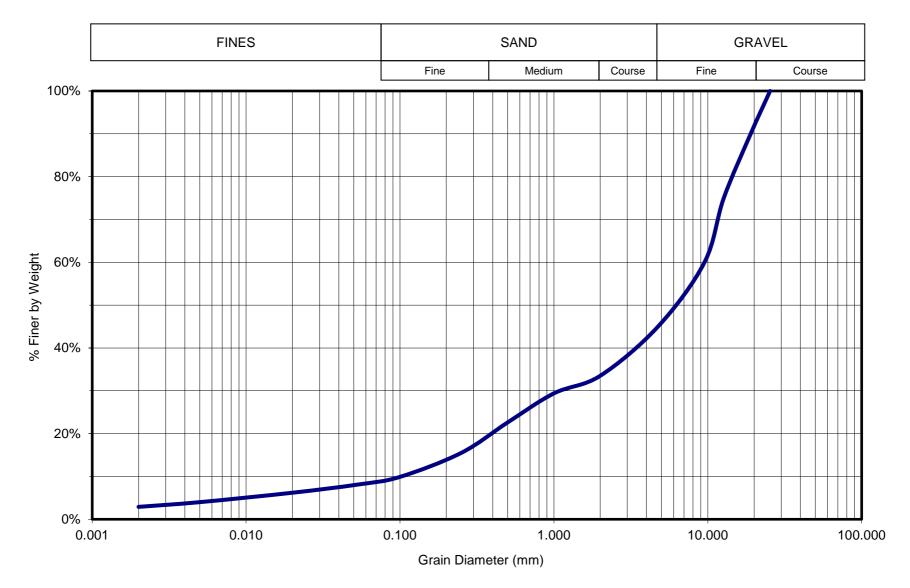
Order Number:	26938
Lab Number:	X161219-103
Received:	12/19/2016
Reported:	12/22/2016

USDA Size Fraction	L.		Per	rcent of W	hole Sample Passing	
<u>Main Fractions</u> Sand Silt Clay	Size (mm) 0.05-2.0 0.002-0.05 <0.002	Percent 75.8 15.7 8.5	<u>Size (mm)</u> 2.00 1.00 0.50 0.25	<u>Sieve #</u> #10 #18 #35 #60	Whole Sample % of Sample Passing 33.5 29.4 22.6	
Sand Fractions Very Coarse Coarse Medium Fine Very Fine	Size (mm) 1.0-2.0 0.5-1.0 0.25-0.5 0.10-0.25 0.05-0.10	Percent 12.3 20.4 21.0 16.6 5.4	0.23 0.10 0.053 0.02 0.005 0.002	#00 #140 #270 20 um 5 um 2 um	15.5 9.9 8.1 6.2 4.0 2.9	
<u>Silt Fractions</u> Coarse Medium Fine	Size (mm) 0.02-0.05 0.005-0.02 0.002-0.005	Percent 5.8 6.6 3.3				

USDA Textural Class: gravelly coarse sandy loam

Gravel Content: (%) 6

66.5



Grain size analysis for SWTP1, 11 to 12'



Prepared For:

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Soil and Plant Nutrient Testing Laboratory

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Sample Information:

Sample ID: SWTP 2 10 to 12'

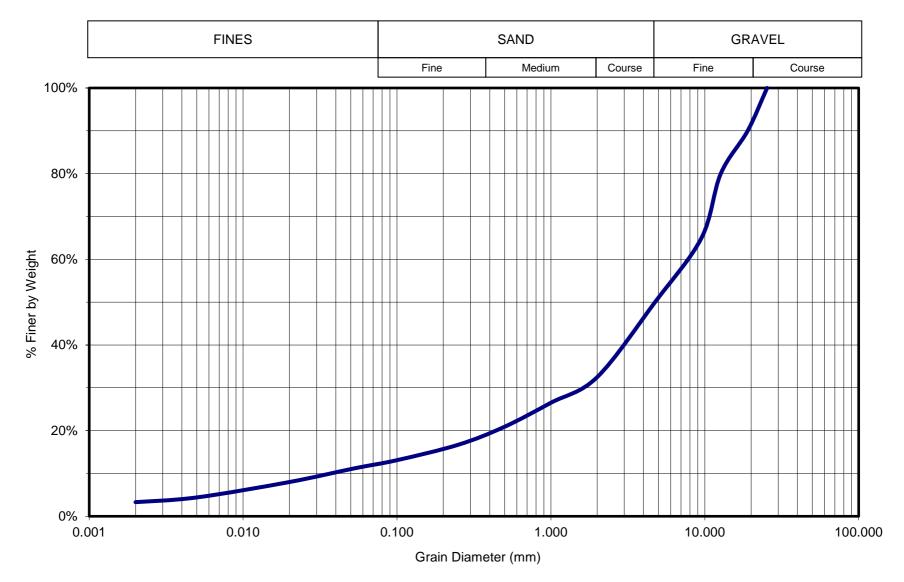
.04
6
6

USDA Size Fraction			<u>Per</u>	rcent of W	hole Sample Passing
Main Fractions Sand	<u>Size (mm)</u> 0.05-2.0	<u>Percent</u> 65.4	<u>Size (mm)</u>	<u>Sieve #</u>	<u>Whole Sample % of</u> <u>Sample Passing</u>
Silt Clay	0.002-0.05 <0.002	24.4 10.2	2.00 1.00 0.50	#10 #18 #35	32.5 26.5 20.9
Sand Fractions	<u>Size (mm)</u>	Percent	0.25 0.10	#60 #140	16.7 13.1
Very Coarse Coarse	1.0-2.0 0.5-1.0	18.3 17.4	0.053 0.02	#270 20 um	11.2 8.0
Medium Fine	0.25-0.5 0.10-0.25	12.8 11.0	0.005 0.002	5 um 2 um	4.4 3.3
Very Fine	0.05-0.10	5.9			
Silt Fractions	<u>Size (mm)</u>	Percent			
Coarse	0.02-0.05	10.0			
Medium	0.005-0.02	11.0			
Fine	0.002-0.005	3.5			

USDA Textural Class: gravelly coarse sandy loam

Gravel Content: (%) 6

67.5



Grain size analysis for SWTP2, 10 to 12'



Prepared For:

Jack Jemsek SWCA Incorporated 15 Research Drive Amherst, MA 01002

jjemsek@swca.com 413-658-2055

Soil and Plant Nutrient Testing Laboratory

203 Paige Laboratory 161 Holdsworth Way University of Massachusetts Amherst, MA 01003 Phone: (413) 545-2311 e-mail: soiltest@umass.edu website: soiltest.umass.edu

Sample Information:

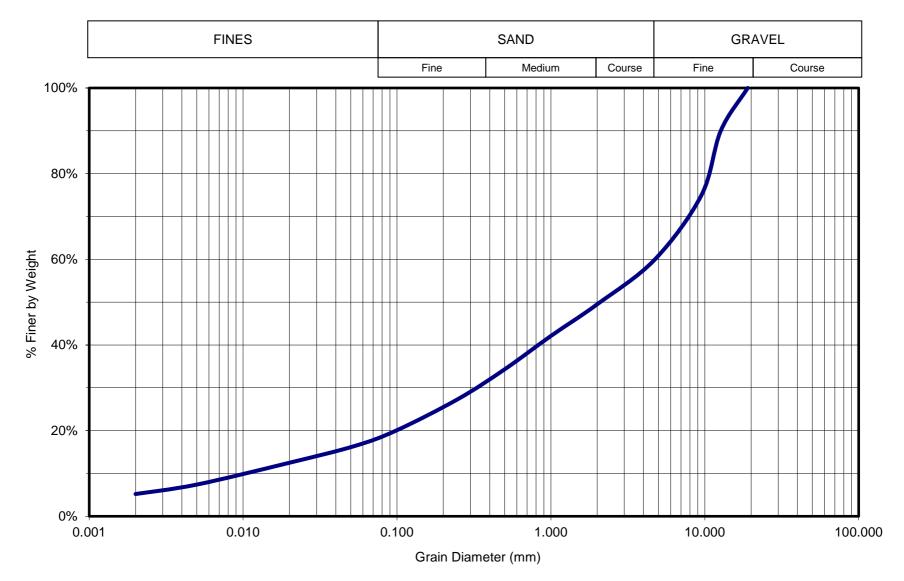
Sample ID: SWTP 3 5 to 6'

Order Number:	26938
Lab Number:	X161219-105
Received:	12/19/2016
Reported:	12/22/2016

USDA Size Fraction			Per	rcent of W	hole Sample Passing
<u>Main Fractions</u> Sand Silt	<u>Size (mm)</u> 0.05-2.0 0.002-0.05	<u>Percent</u> 66.9 22.5	<u>Size (mm)</u> 2.00	<u>Sieve #</u> #10	Whole Sample % of Sample Passing 49.5
Clay	<0.002	10.6	1.00 0.50 0.25	#18 #35 #60	42.1 34.3 27.4
Sand Fractions Very Coarse Coarse	<u>Size (mm)</u> 1.0-2.0 0.5-1.0	<u>Percent</u> 15.1 15.7	0.10 0.053	#140 #270	20.1 16.4
Medium Fine	0.25-0.5 0.10-0.25	14.0 14.8	0.02 0.005 0.002	20 um 5 um 2 um	12.5 7.4 5.2
Very Fine	0.05-0.10	7.4			
Silt Fractions Coarse Medium	<u>Size (mm)</u> 0.02-0.05 0.005-0.02	<u>Percent</u> 7.9 10.1			
Fine	0.002-0.005	4.5			

USDA Textural Class: gravelly coarse sandy loam

Gravel Content: (%) 50.5



Grain size analysis for SWTP3, 5 to 6'

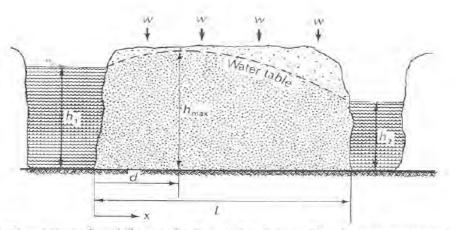
ATTACHMENT C

Groundwater Mounding Worksheets

-1

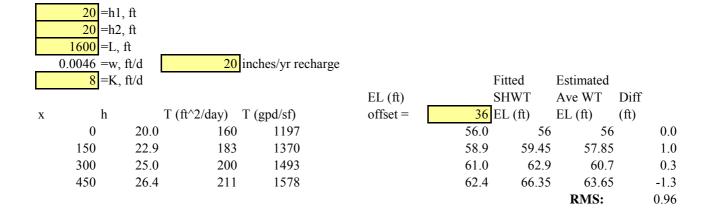


X-h Profile using Dupuit-Forcheimier Equation for Unconfined Flow





$$h^{2} = h_{1}^{2} - \frac{(h_{1}^{2} - h_{2}^{2})x}{L} + \frac{w}{K}(L - x)x$$



This spreadsheet will calculate the height of a groundwater mound beneath a stormwater infiltration basin. More information can be found in the U.S. Geological Survey Scientific Investigations Report 2010-5102 "Simulation of groundwater mounding beneath hypothetical stormwater infiltration basins".

The user must specify infiltration rate (R), specific yield (Sy), horizontal hydraulic conductivity (Kh), basin dimensions (x, y), duration of infiltration period (t), and the initial thickness of the saturated zone (hi(0), height of the water table if the bottom of the aquifer is the datum). For a square basin the half width equals the half length (x = y). For a rectangular basin, if the user wants the water-table changes perpendicular to the long side, specify x as the short dimension and y as the long dimension. Conversely, if the user wants the values perpendicular to the short side, specify y as the short dimension, x as the long dimension. All distances are from the center of the basin. Users can change the distances from the center of the basin at which water-table aquifer thickness are calculated.

Cells highlighted in yellow are values that can be changed by the user. Cells highlighted in red are output values based on user-specified inputs. The user MUST click the blue "Re-Calculate Now" button each time ANY of the user-specified inputs are changed otherwise necessary iterations to converge on the correct solution will not be done and values shown will be incorrect. Use consistent units for all input values (for example, feet and days)

Input Values inch/hour feet/day 0.9200 R Recharge (infiltration) rate (feet/day) 0.67 1.33
0.9200 B Becharge (infiltration) rate (feet/day) $0.67 1.33$
0.300 Sy Specific yield, Sy (dimensionless, between 0 and 1)
8.00 K Horizontal hydraulic conductivity, Kh (feet/day)* 2.00 4.00 34.500 x 1/2 length of basin (x direction in feet) 1/2 length of basin (x direction in feet)
34.500 x 1/2 length of basin (x direction, in feet) (USG SIR 2010-5102), vertical soil permeability
25.000 y 1/2 width of basin (y direction, in feet) hours days (fr/d) is assumed to be one-tenth horizontal
4.000 t duration of infiltration period (days) 36 1.50 hydraulic conductivity (ft/d).
23.000 hi(0) initial thickness of saturated zone (feet)

maximum thickness of saturated zone (beneath center of basin at end of infiltration period)

maximum groundwater mounding (beneath center of basin at end of infiltration period)



Ground- Distance from water center of basin Mounding, in in x direction, in feet feet 3.435 0 3.416 5 3.356 10

20

40 80

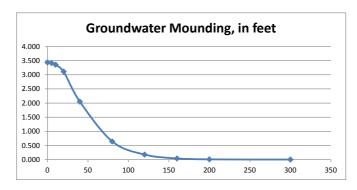
120

160

200

300

Re-Calculate Now



Disclaimer

0.179

This spreadsheet solving the Hantush (1967) equation for ground-water mounding beneath an infiltration basin is made available to the general public as a convenience for those wishing to replicate values documented in the USGS Scientific Investigations Report 2010-5102 "Groundwater mounding beneath hypothetical stormwater infiltration basins" or to calculate values based on user-specified site conditions. Any changes made to the spreadsheet (other than values identified as user-specified) after transmission from the USGS could have unintended, undesirable consequences. These consequences could include, but may not be limited to: erroneous output, numerical instabilities, and violations of underlying assumptions that are inherent in results presented in the accompanying USGS published report. The USGS assumes no responsibility for the consequences of any changes made to the spreadsheet. If changes are made to the spreadsheet, the user is responsible for documenting the changes and justifying the results and conclusions.

Area			
		RESULTS	
Perc (mpi)	1.66	delta h(max) ft	0.5
L (ft)	52	Distance to wetland:	<mark>170</mark>
W (ft)	25	Image Source Effect	0.3
Area (SF)	1300	revised delta h(max)	0.2
Loading			
GPD	1200	Est. SHWT EI (ft)	<mark>58.0</mark>
GPD/SF	0.92	EL + delta h(max) (ft)	58.2
FT/D	0.12	Min Bed Bottom El (ft)	62.2

SDS #1

Western

1

SYSTEM #1 BUILDING/FACILITY: EIGHT (8) ELDERY DWELLING UNITS NUMBER OF BEDROOOMS: TWO (2) BEDROOMS PER UNIT, 16 TOTAL DESIGN FLOW RATE: 150 GPD PER TWO BEDROOM ELDERLY UNIT PERCOLATION RATE: 1.66 MPI (PERC-1 AT TP-2) TOTAL DESIGN FLOW: 1,200 GPD LEACHING AREA REQUIREMENTS (GPD/SF): 0.60 GPD PER SF (1,200 GPD)/(0.6 GPD/SF)=2,000 SF PER TITLE 5 REQUIREMENT (2,000 SF) × (40% REDUCTION FOR PRESBY SYSTEM) = 1,200 SF MINIMUM SAND BED AREA: (8 UNITS)x(150 GPD)/(1 GPD/SF FOR PRESBY SYSTEM) = 1,200 SF LEACHING PROVIDED IN THE DESIGN; PRESBY SYSTEM USE 12 ROWS, 50' LONG PIPES, WITH 2' SPACING 3 ROWS OF PIPES ARE CONNECTED AT END TO MAKE 4 TREATMENT SECTIONS LEACH FIELD 25' x 52' = 1.300 SF (BOTTOM AREA ONLY) REQUIRED PIPE LENGTH IS 546 L.F.; DESIGN 600 LF
Input Values
0.1200 R Recharge (infiltration) rate (feet/day)
0.300 Sy Specific yield, Sy (dimensionless, between 0 and 1)

0.300	Sy	Specific yield, Sy (dimensionless, between 0 and 1)	
8.00	К	Horizontal hydraulic conductivity, Kh (feet/day)*	
26.000	x	1/2 length of basin (x direction, in feet)	
12.500	У	1/2 width of basin (y direction, in feet)	
365.000	t	duration of infiltration period (days)	
23.000	hi(0)	initial thickness of saturated zone (feet)	

Central Area	SDS #3		
		RESULTS	
Perc (mpi)	1.00	delta h(max) ft	0.8
L (ft)	72	Distance to wetland:	240
W (ft)	25	Image Source Effect	0.3
Area (SF)	1800	revised delta h(max)	0.5
Loading			
GPD	1800	Est. SHWT EI (ft)	60.8
GPD/SF	1.00	EL + delta h(max) (ft)	61.3
FT/D	0.13	Min Bed Bottom El (ft)	65.3

SYSTEM #3

SYSTEM #3
BUILDING/FACILITY:TWELVE (12) ELDERY DWELLING UNITS
NUMBER OF BEDROOOMS:TWO (2) BEDROOMS PER UNIT, 24 TOTAL
DESIGN FLOW RATE: 150 GPD PER TWO BEDROOM ELDERLY UNIT
PERCOLATION RATE: 1.00 MPI (PERC-4 AT TP-5)
TOTAL DESIGN FLOW: 1,800 GPD
LEACHING AREA REQUIREMENTS (GPD/SF): (1.800 GPD)/(0.6 GPD/SF)=3,000 SF PER TITLE 5 REQUIREMENT
(3,000 SF) x (40% REDUCTION FOR PRESBY SYSTEM) = 1,800 SF
MINIMUM SAND BED AREA; (12 UNITS)x(150 GPD)/(1 GPD/SF FOR PRESBY SYSTEM) = 1,800 SF
LEACHING PROVIDED IN THE DESIGN: PRESBY SYSTEM USE 12 ROWS, 70' LONG PIPES, WITH 2' SPACING
3 ROWS OF PIPES ARE CONNECTED AT END TO MAKE 4 TREATMENT SECTIONS
LEACH FIELD $25' \times 72' = 1.800$ SF (BOTTOM AREA ONLY)
REQUIRED PIPE LENGTH IS 818 L.F.; DESIGN 840 LF

Input Values			
0.1300	R	Recharge (infiltration) rate (feet/day)	
0.300	Sy	Specific yield, Sy (dimensionless, between 0 and 1)	
8.00	К	Horizontal hydraulic conductivity, Kh (feet/day)*	
36.000	x	1/2 length of basin (x direction, in feet)	
12.500	У	1/2 width of basin (y direction, in feet)	
365.000	t	duration of infiltration period (days)	
23.000	hi(0)	initial thickness of saturated zone (feet)	

Eastern Area	SDS #2				
		RESULTS			
Perc (mpi)	7.33	delta h(max) ft	0.7		
L (ft)	62	Distance to wetland:	<mark>145</mark>		
W (ft)	25	Image Source Effect	0.4		
Area (SF)	1550	revised delta h(max)	0.3		
Loading					
GPD	1500	Est. SHWT El (ft)	<mark>64.0</mark>		
GPD/SF	0.97	EL + delta h(max) (ft)	64.3		
FT/D	0.13	Min Bed Bottom El (ft)	68.3		
SYSTEM #2	2				
BUILDING/F		TEN (10) ELDERY DWELLING	G UNITS PER UNIT, 20 TOTAL		
NUMBER OF BEDROOOMS: <u>TWO (2) BEDROOMS PER UNIT, 20 TOTAL</u> DESIGN FLOW RATE: 150 GPD PER TWO BEDROOM ELDERLY UNIT					
PERCOLATION RATE: 7.33 MPI (PERC-6 AT TP-10)					
TOTAL DESIG	on neon.	1,500 GPD			
(1.500 G	REA REQU PD)/(0.6	IIREMENTS (GPD/SF): GPD/SF)=2,500 SF PER TITLE 5	5 REQUIREMENT		
(2.500 SI	F) x (40%	REDUCTION FOR PRESBY SYSTI	EM) = 1,500 SF		
MINIMUM SA	ND BED	AREA: PD)/(1_GPD/SF_FOR_PRESBY_S`	YSTEM) = 1,500 SF		
LEACHING P PRESBY SY	ROVIDED	IN THE DESIGN: 12 ROWS, 60' LONG PIPES, W	ITH 2' SPACING		
3 ROWS OF	PIPES A	RE CONNECTED AT END TO MAK	E 4 TREATMENT SECTIONS		
REQUIRED F	PIPE LENG	52' = 1,550 SF (BOTTOM AREA TH IS 682 L.F.: DESIGN 720 LF	ONLY)		

Input Values			
0.1300	R	Recharge (infiltration) rate (feet/day)	
0.300	Sy	Specific yield, Sy (dimensionless, between 0 and 1)	
8.00	К	Horizontal hydraulic conductivity, Kh (feet/day)*	
31.000	x	1/2 length of basin (x direction, in feet)	
12.500	У	1/2 width of basin (y direction, in feet)	
365.000	t	duration of infiltration period (days)	
23.000	hi(0)	initial thickness of saturated zone (feet)	



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